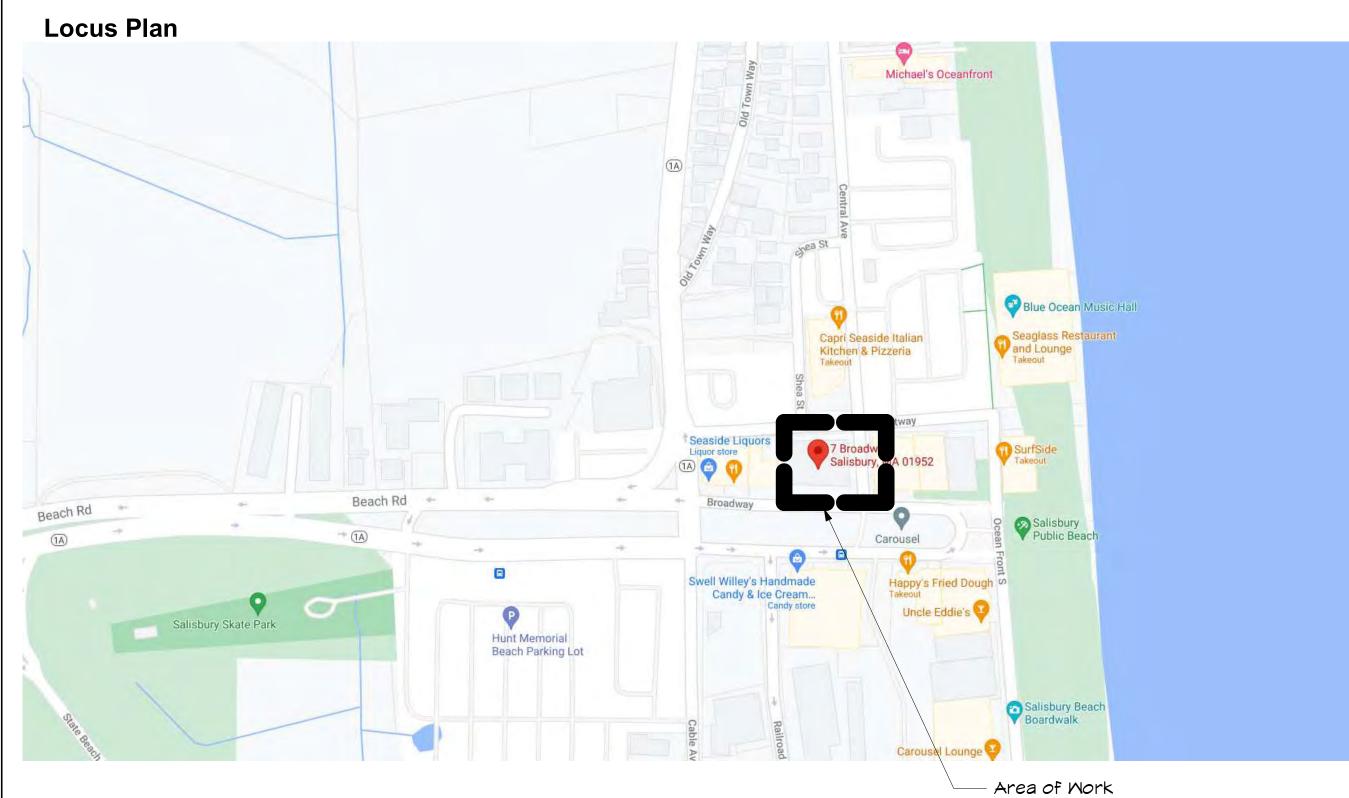
Project Summary

These drawings depict the new construction of an approx. 6641 s.f. octagonal timber framed Carousel House building with attached support and commercial/retail spaces. The existing non-conforming one-story building that is approximately 9992 s.f. will be demolished and removed. The proposed building is within a Coastal A zone and will be elevated on an open pile foundation with a minimum of 2 feet clearance from grade to the lowest horizontal structure. Handicap accessibly for entrance/egress will be provided by exterior ramps. Two staff parking spaces are to be provided on site that will be accessed with an existing curb cut from Driftway.

Building Aerial View







7 Broadway, Salisbury, MA

11/2/21

PROGRESS SET

DRAWING LIST

		A100	Architectural Site Plan	
General P	roject Information	A111	First Floor Plan	Structural
T-1	Title Sheet	A121	Roof Plan	S000
T-2	Code Analysis	AR111	Reflected Ceiling Plan	S100
		A201	South Elevation & Building Section	S101
Civil		A202	North & East Elevations	S102
C1	Existing Conditions Plan	A401	Interior Elevations	S200
C2	Site Plan	A501	Buidling Details	S201
C3	Site Details	A502	Building Details	
		A503	Building Details	Lighting
Architectu	ral	A504	Stair Section Detail	L1
AD100	Demolition Plan	A601	Door Schedule & Types	L2
A001	Site Diagram	A602	Window Schedule & Types	
A002	Construction Work Area Plan	A603	Partition Schedule	



Rendered View from Broadway

OWNER/CLIENT

Salisbury Beach Partnership

98 Elm Street Salisbury, MA 01952

ARCHITECT



978-750-9062

gienapparchitects.com

20 Conant Street

Danvers, MA 01923

<u>CIVIL</u>

Millennium Engineering, Inc.

62 Elm Street, Salisbury, MA 01952 978-463-8980 13 Hampton Road, Exeter, NH 03833 603-778-0528

Area of Work

General Notes Foundation / Pile Layout Plan First Floor Framing Plan Partial Floor Framing Plans Details and Section Details and Sections

Lighting Plan, Schedule, & Renderings Photometric Diagram

STRUCTURAL

TLH Consulting

505 Middlesex Turnpike #14 Billerica, MA 01820 978-362-1804

HVAC / PLUMBING / FIRE PROTECTION

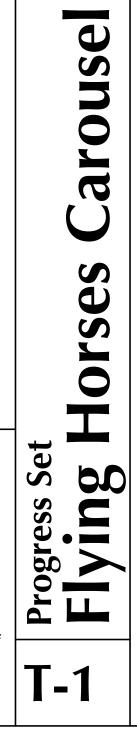
Dry Air Systems, Inc.

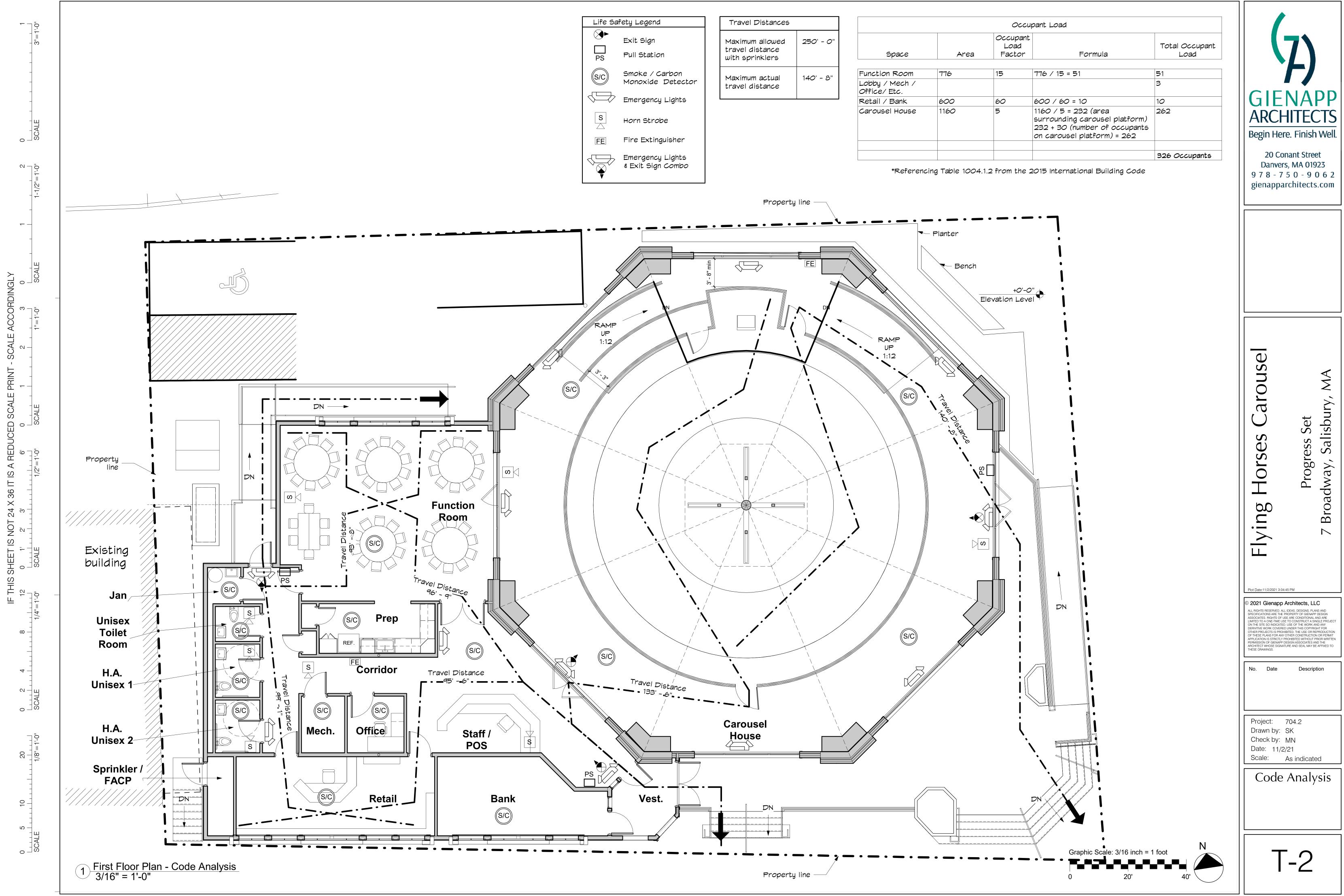
62 Forest Ridge Drive Rowley, MA 01969 978-948-2151

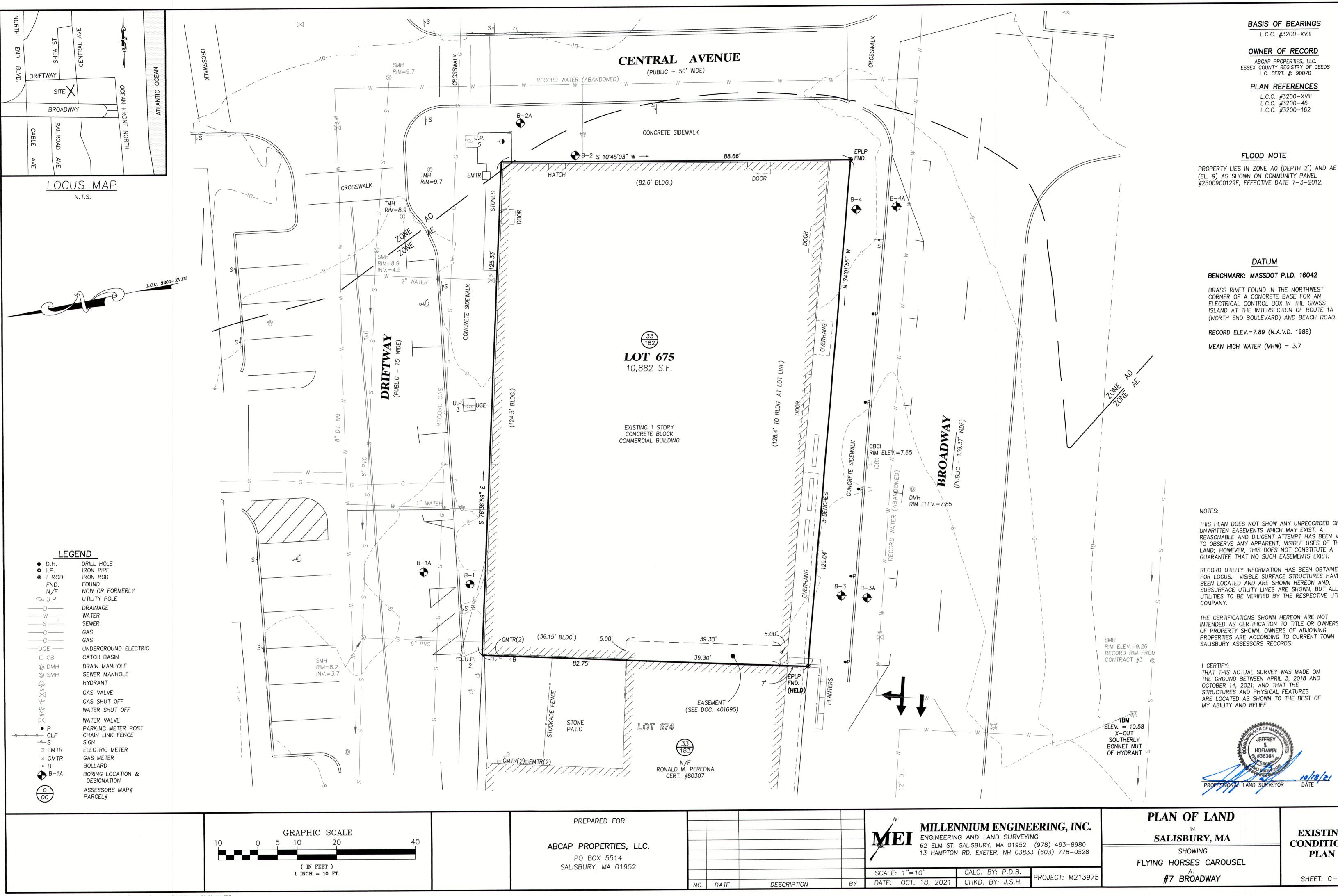
<u>Lighting</u>

Boston Light Source

64 Commercial Wharf Boston, MA 02110 617-788-2400







E:\sdskproj-2021\M213975\dwg\F-EXISTING CONDITIONS (10-18-21).dwg 10/18/2021 11:39:09 AM EDT

PROPERTY LIES IN ZONE AO (DEPTH 2') AND AE

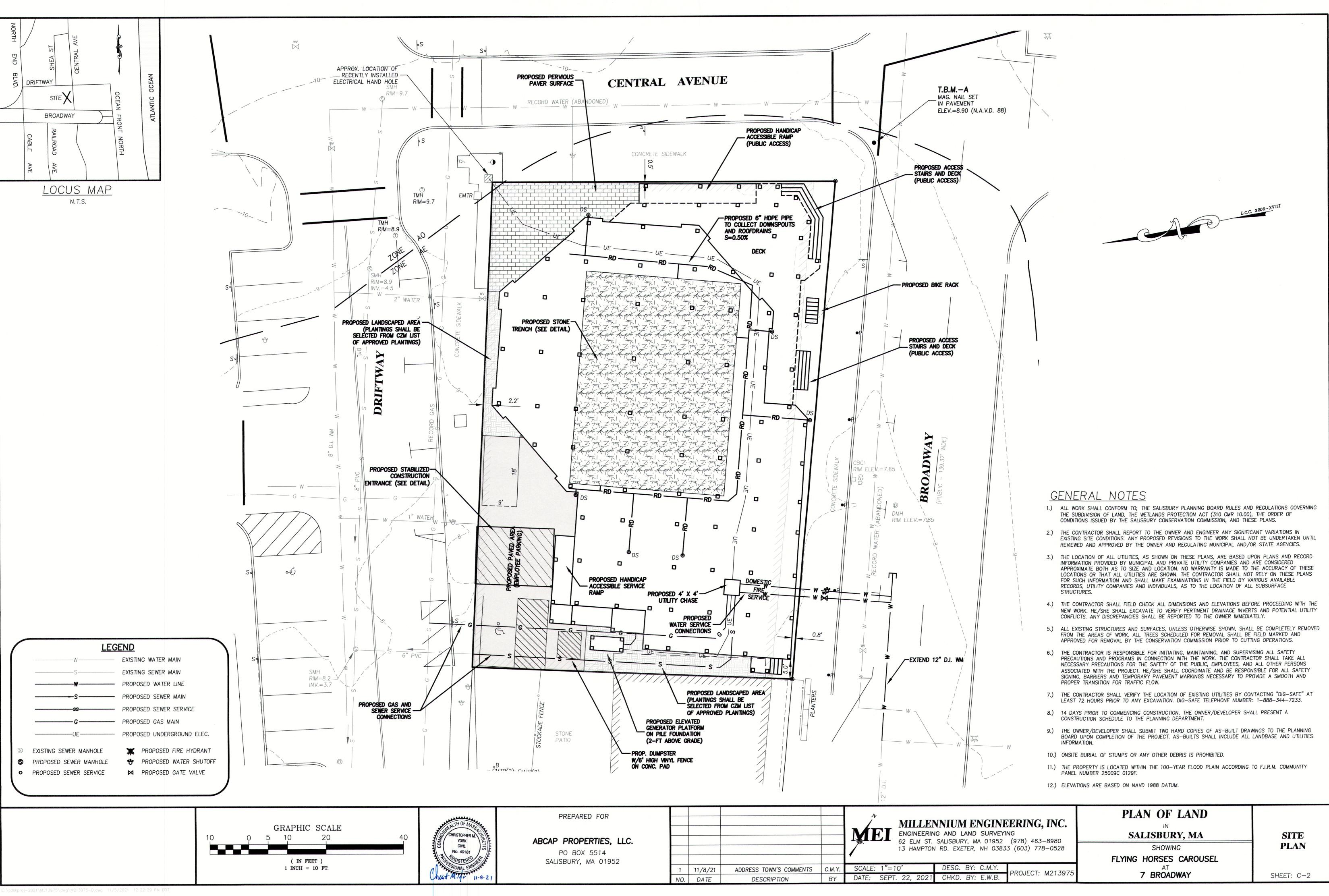
ELECTRICAL CONTROL BOX IN THE GRASS ISLAND AT THE INTERSECTION OF ROUTE 1A

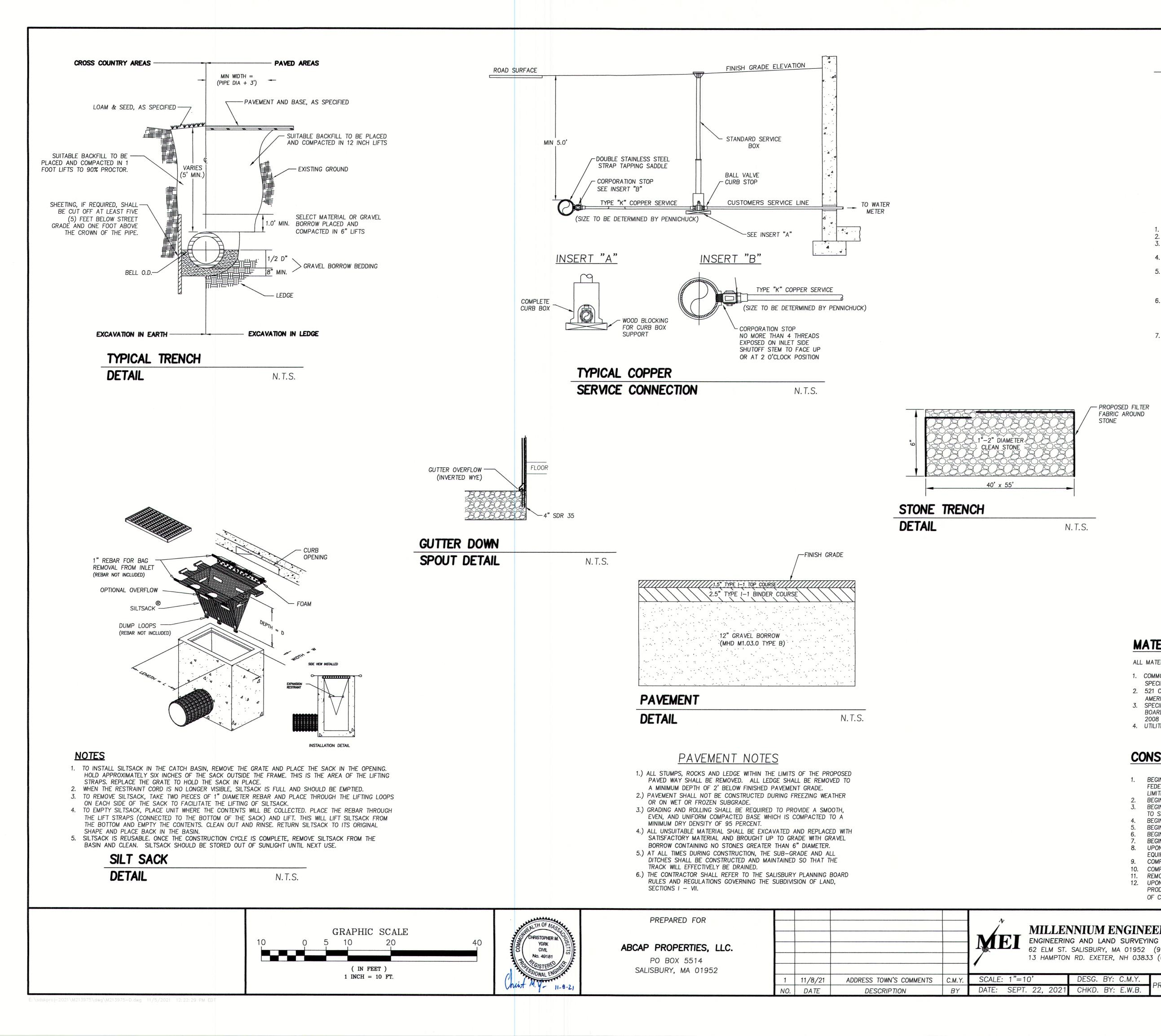
THIS PLAN DOES NOT SHOW ANY UNRECORDED OR UNWRITTEN EASEMENTS WHICH MAY EXIST. A REASONABLE AND DILIGENT ATTEMPT HAS BEEN MADE TO OBSERVE ANY APPARENT, VISIBLE USES OF THE LAND; HOWEVER, THIS DOES NOT CONSTITUTE A GUARANTEE THAT NO SUCH EASEMENTS EXIST.

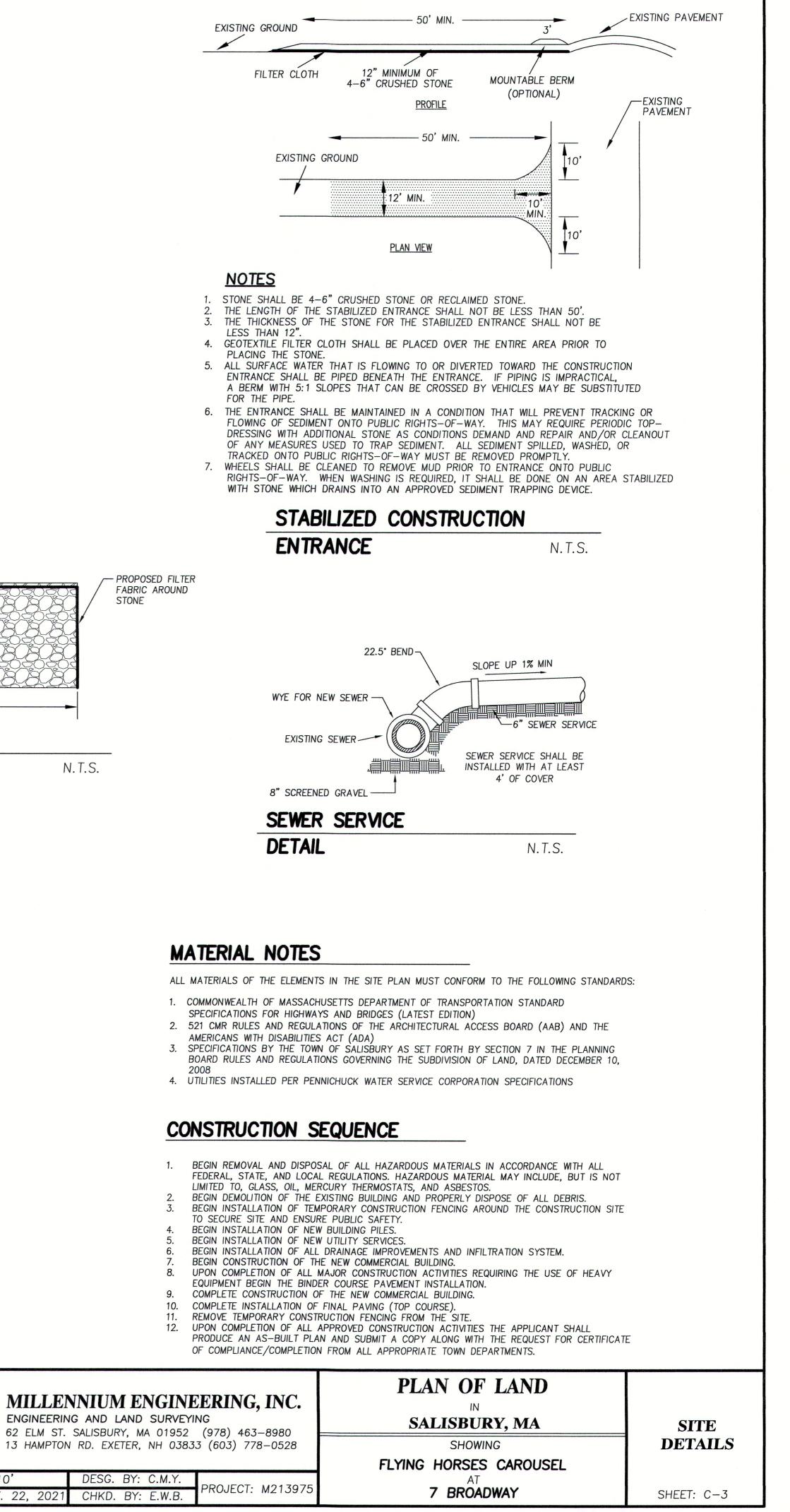
RECORD UTILITY INFORMATION HAS BEEN OBTAINED FOR LOCUS. VISIBLE SURFACE STRUCTURES HAVE BEEN LOCATED AND ARE SHOWN HEREON AND, SUBSURFACE UTILITY LINES ARE SHOWN, BUT ALL UTILITIES TO BE VERIFIED BY THE RESPECTIVE UTILITY

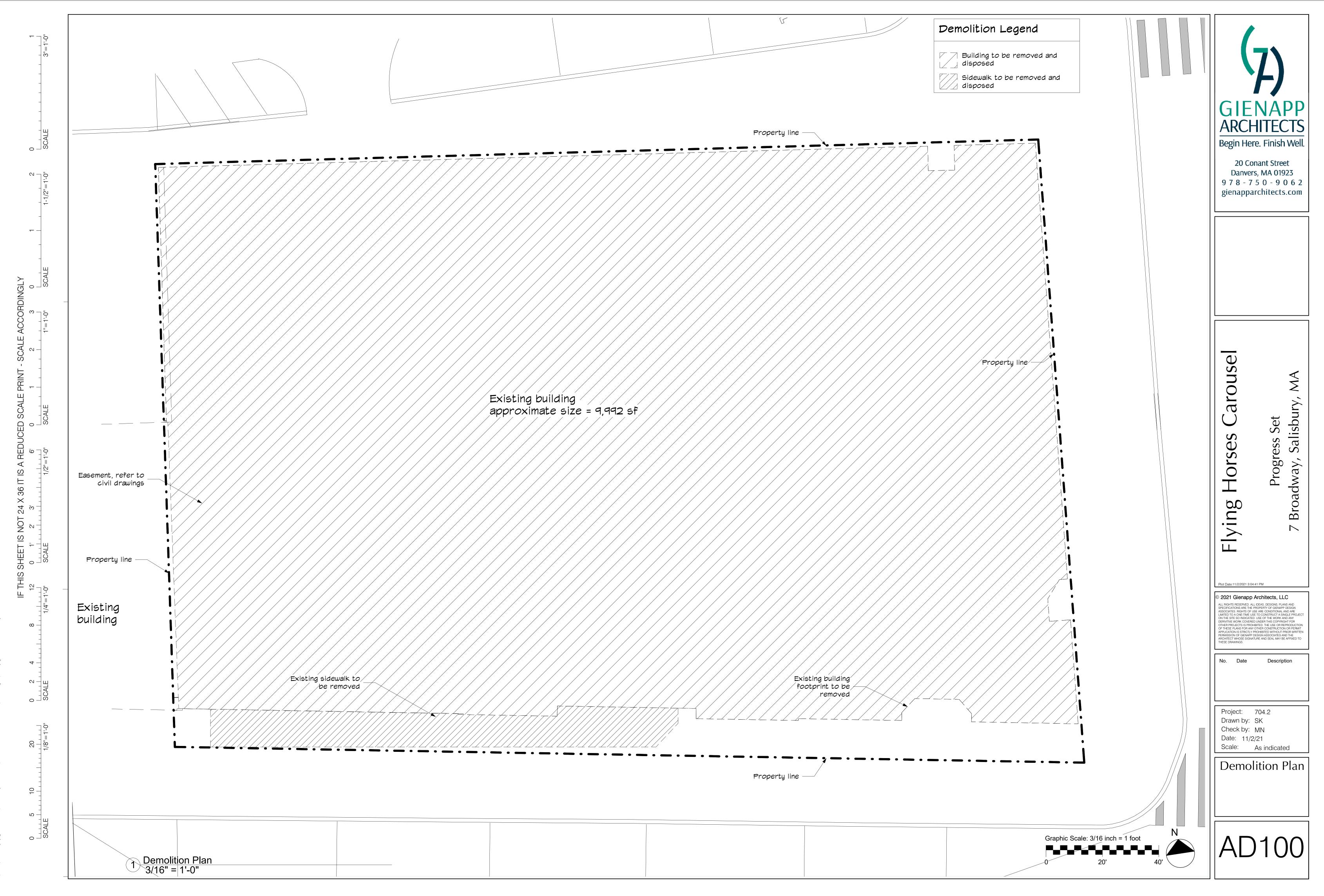
INTENDED AS CERTIFICATION TO TITLE OR OWNERSHIP OF PROPERTY SHOWN. OWNERS OF ADJOINING PROPERTIES ARE ACCORDING TO CURRENT TOWN OF

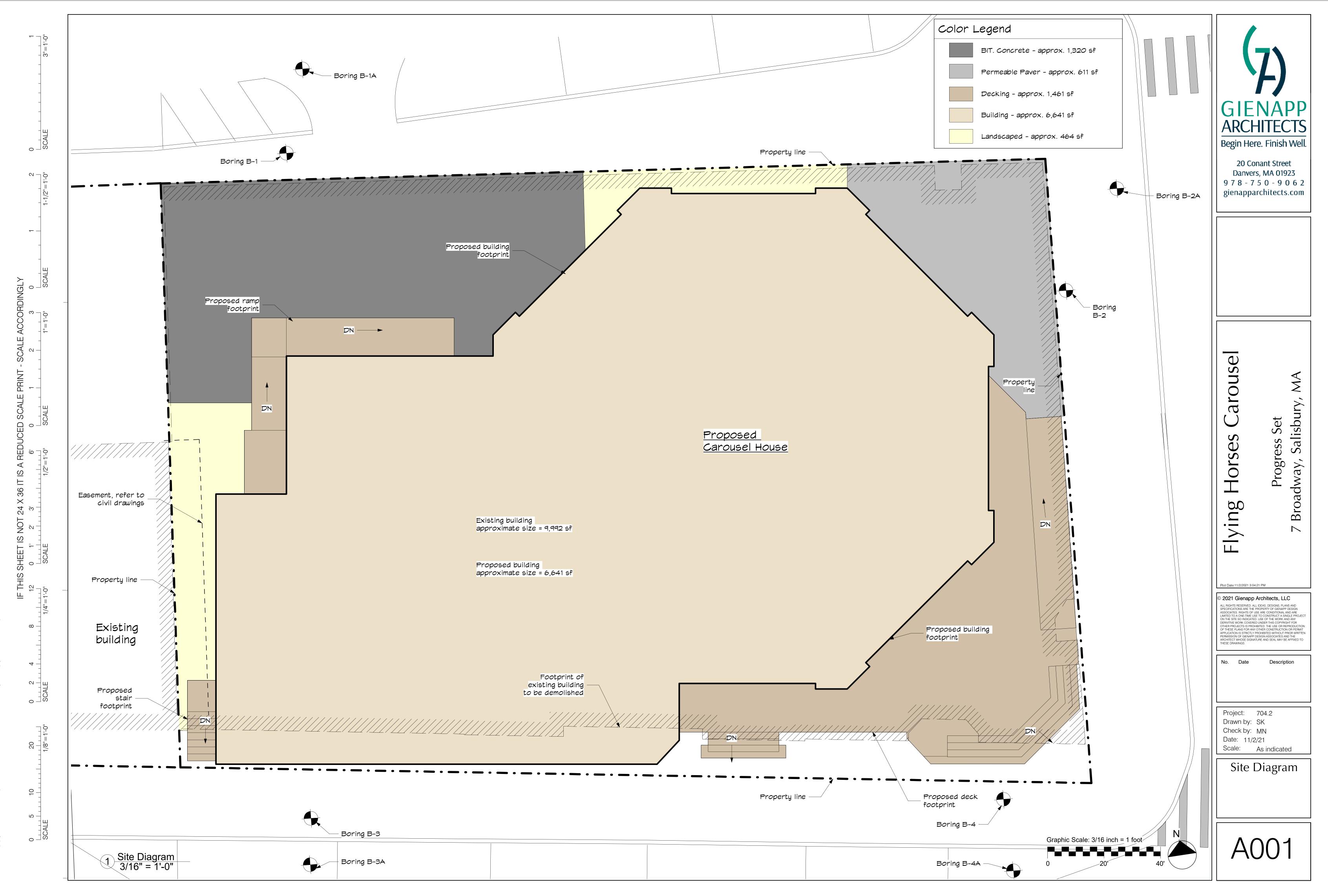
RING ST. S	NIUM ENGINE and land surveyi alisbury, ma 01952 rd. exeter, nh 0383	NG (978) 463-8980	PLAN OF LAND IN SALISBURY, MA SHOWING FLYING HORSES CAROUSEL	EXISTING CONDITIONS PLAN
T	CALC. BY: P.D.B.	DD0 (507, M217075	AT	
1	CHKD. BY: J.S.H.	PROJECT: M213975	#7 BROADWAY	SHEET: C-1



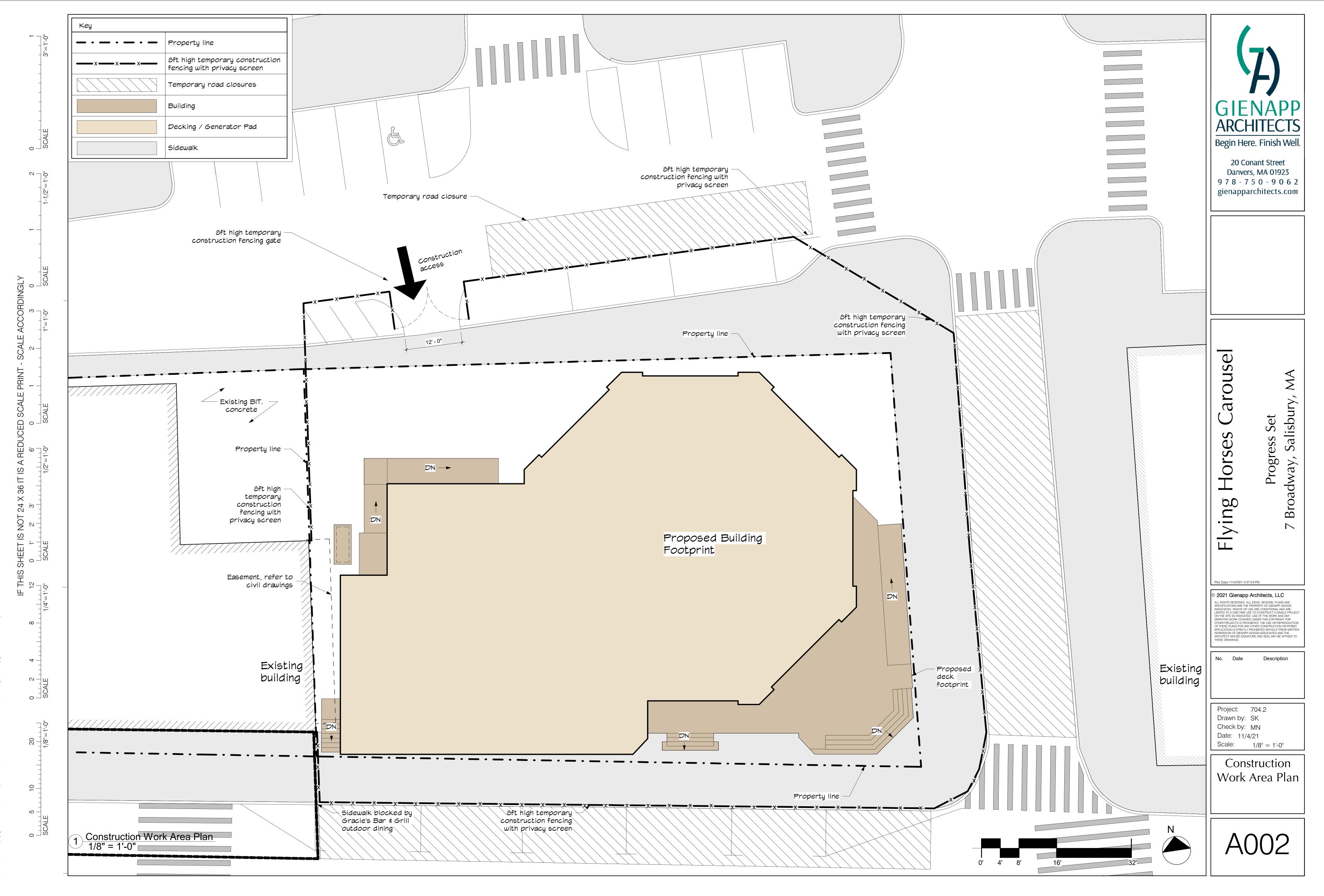




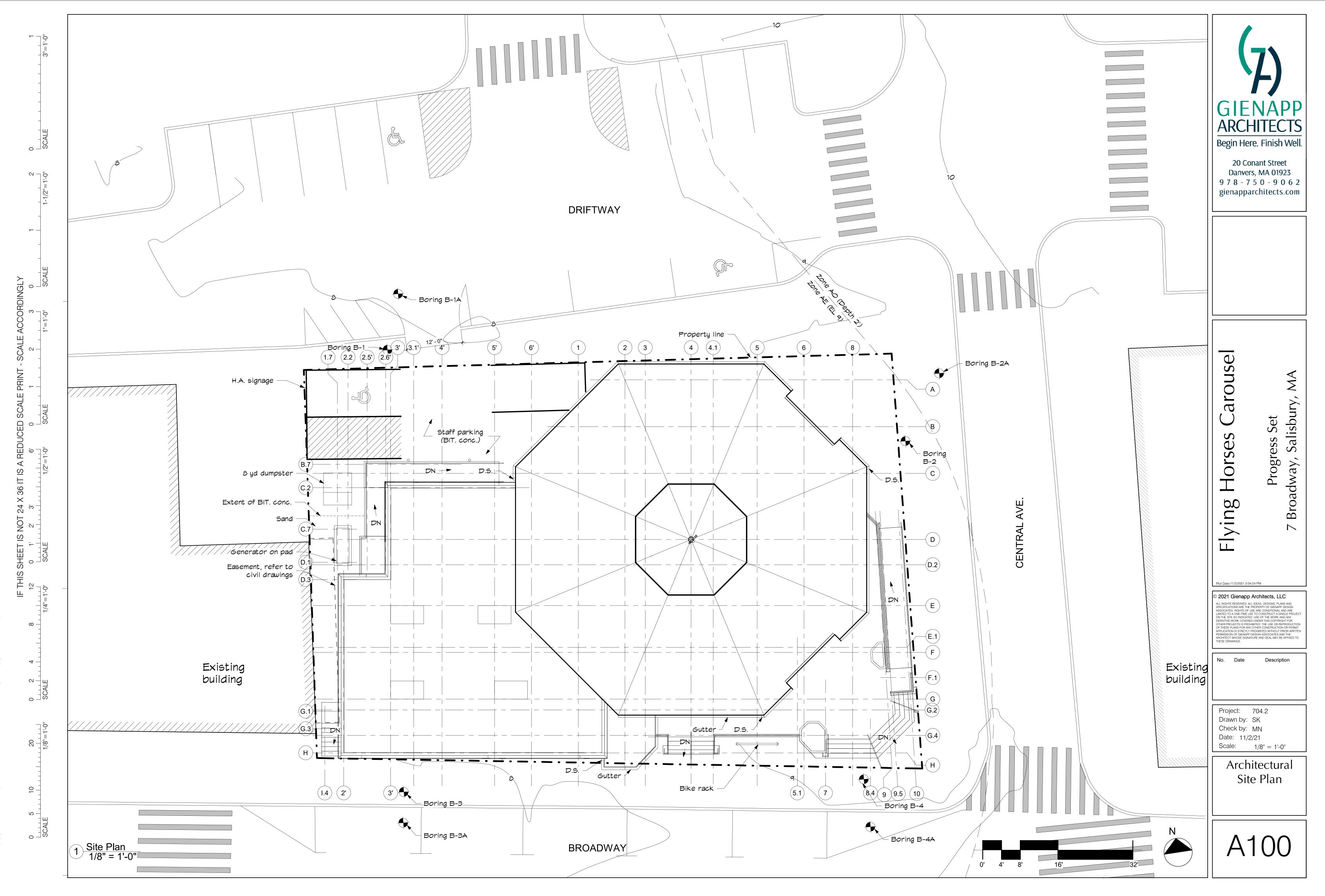


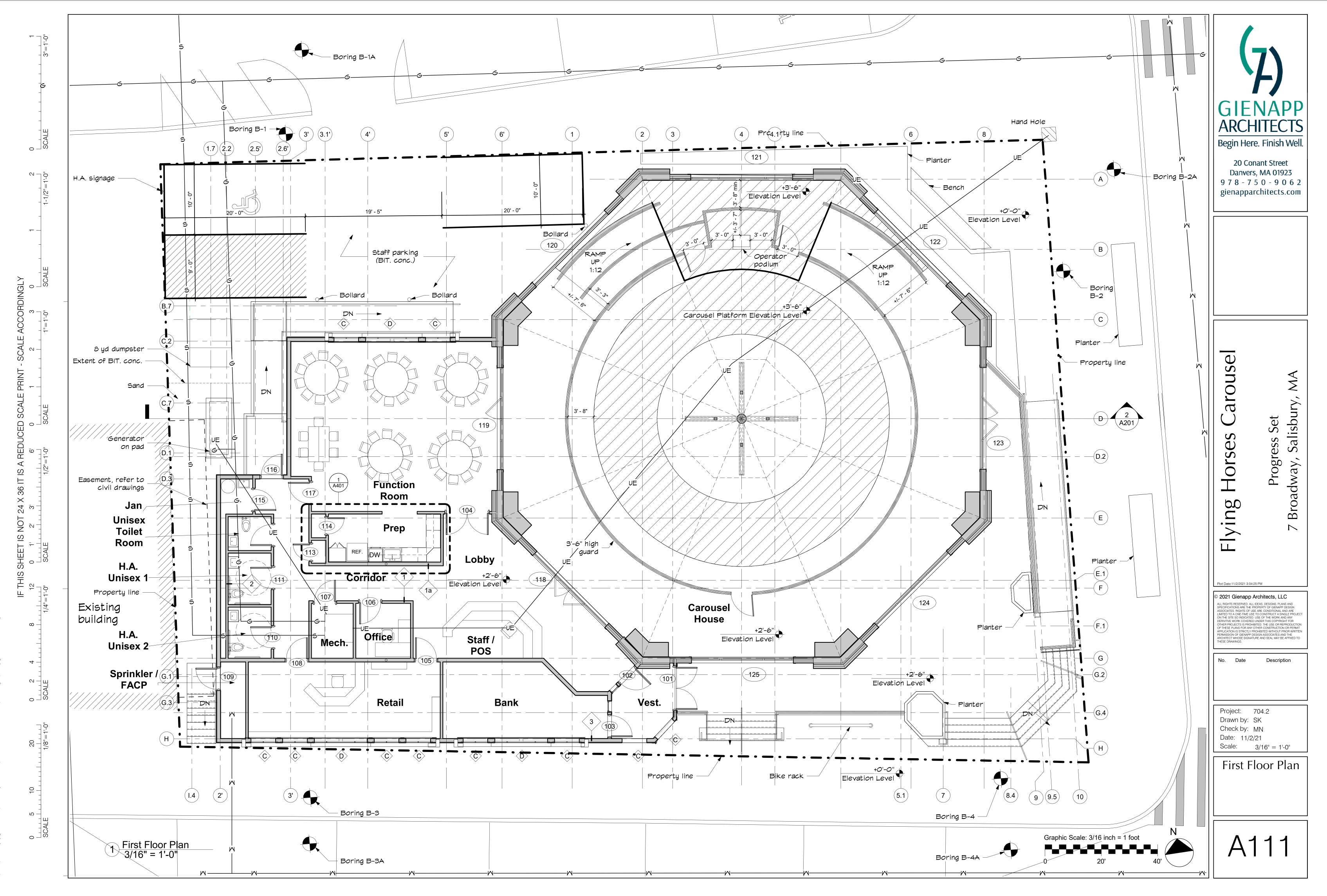


750/704 Salisbury Flying Horses Carousel/704.2 Salisbury Carousel House/704.2 DRAWINGS AND SPECIFICATIONS/704.2 Working Set/704.2 Flying Horses Carousel 7 Broadway 10272021.nt

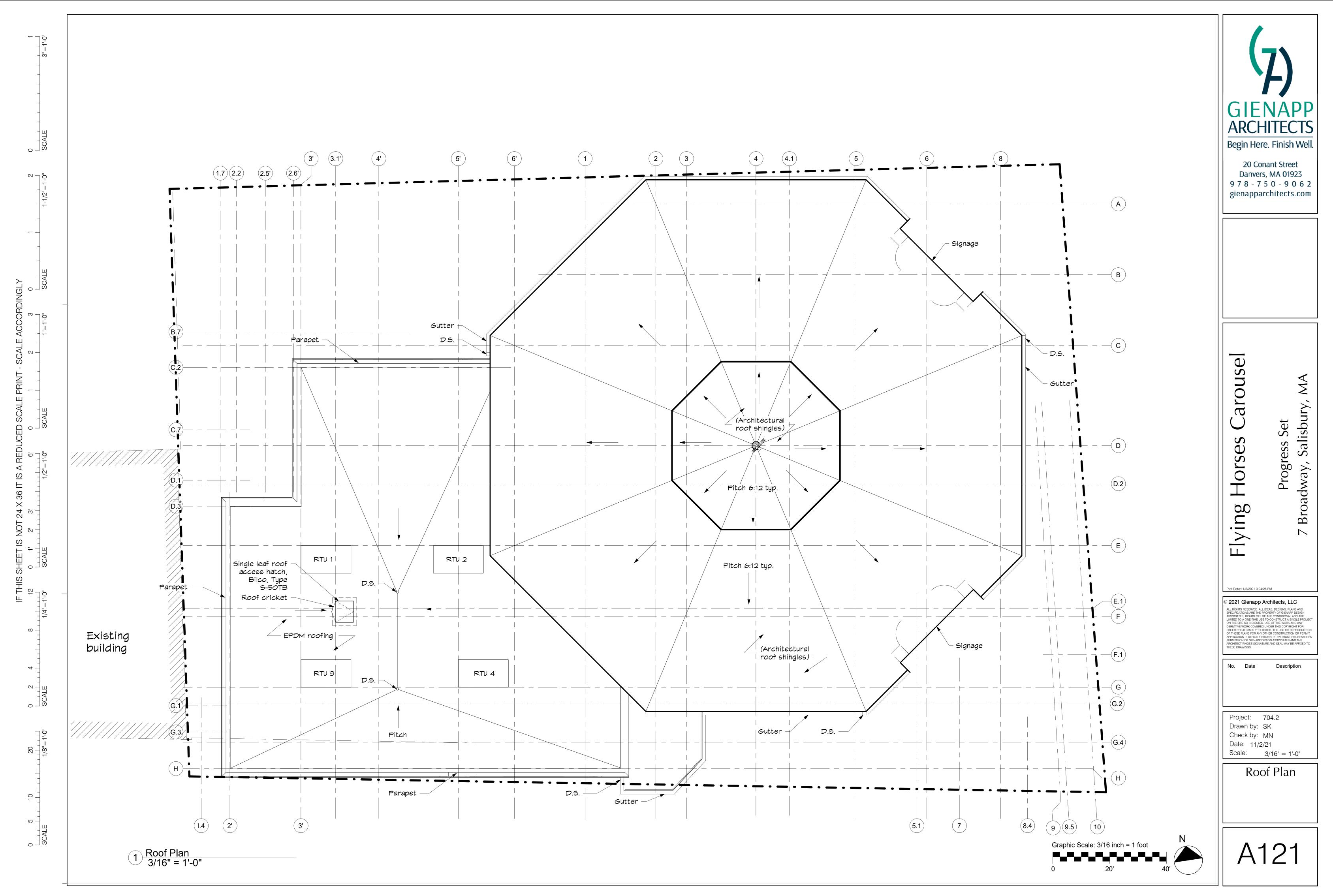


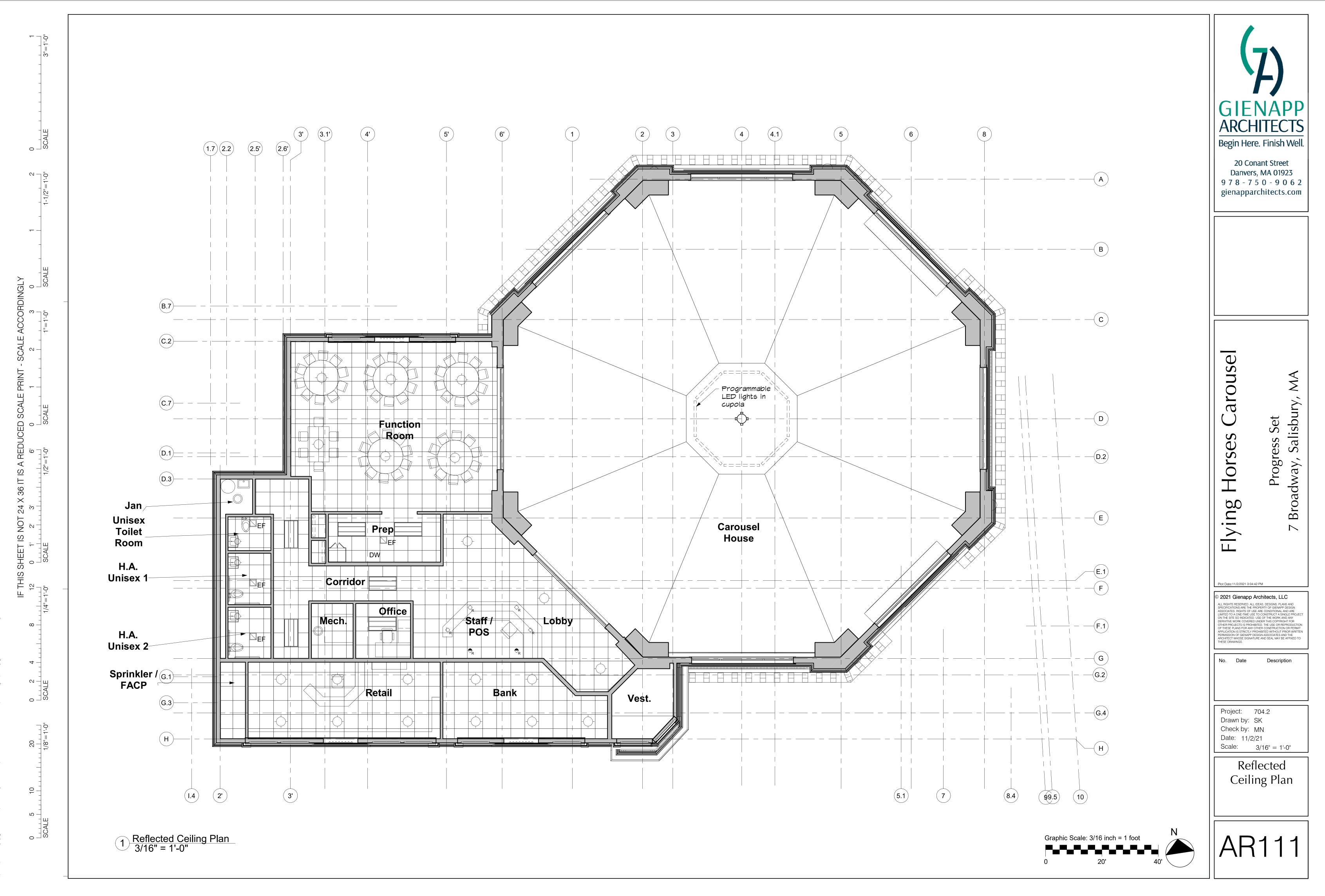
50/704 Salisbury Flying Horses Carousel/704.2 Salisbury Carousel House/704.2 DRAWINGS AND SPECIFICATIONS/704.2 Working Set/704.2 Flying Horses Carousel 7 Broadway 10272021.nt

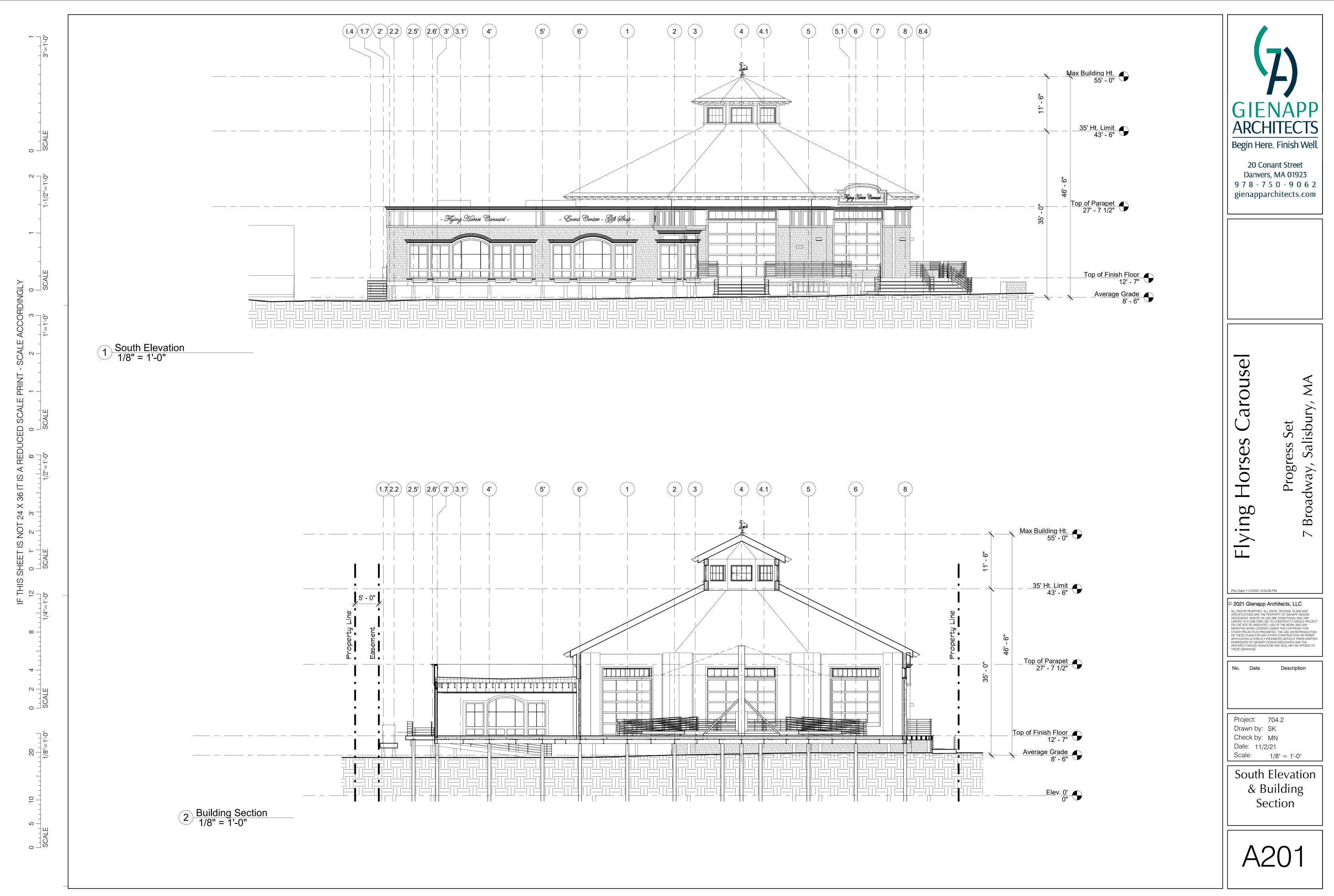






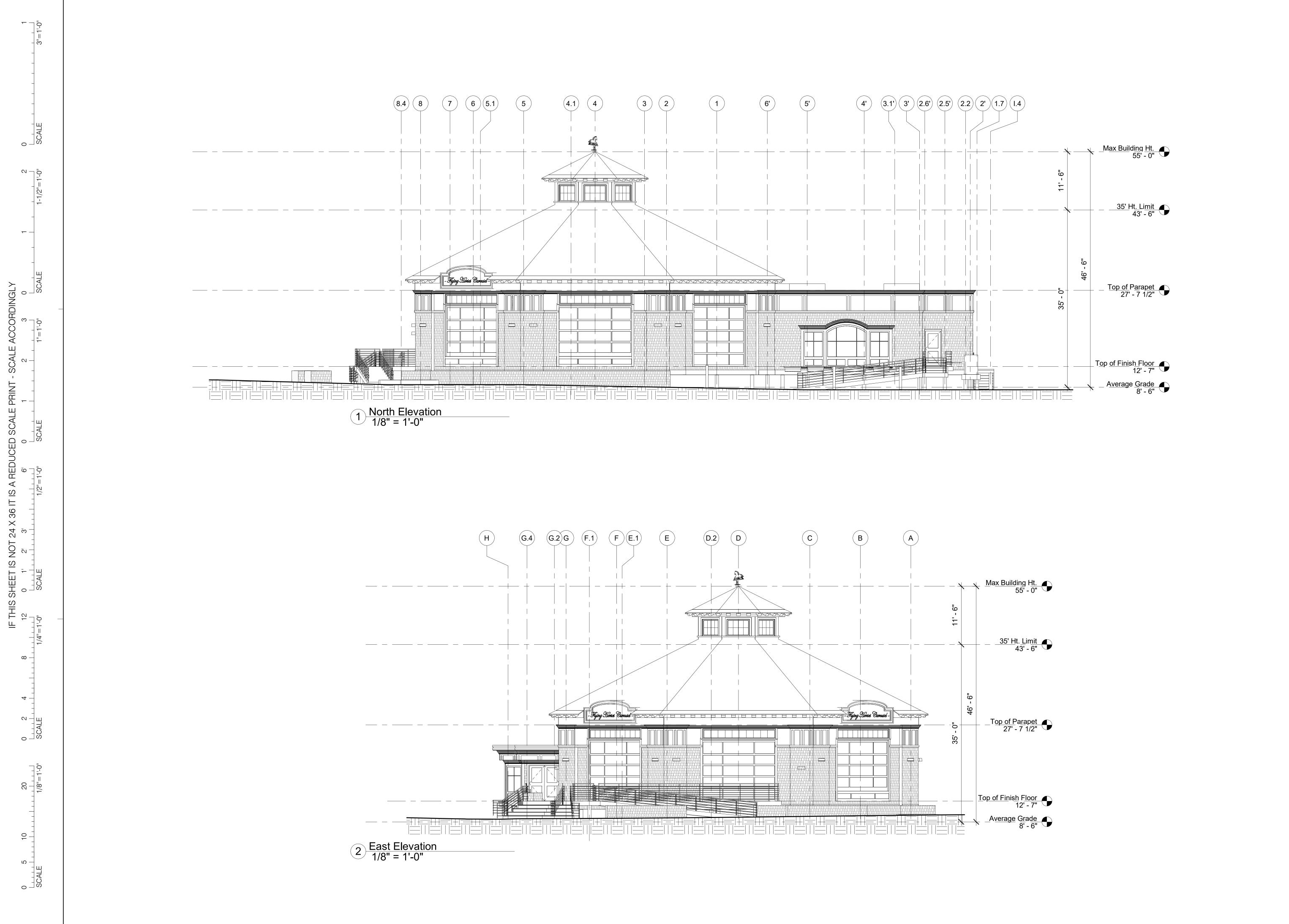






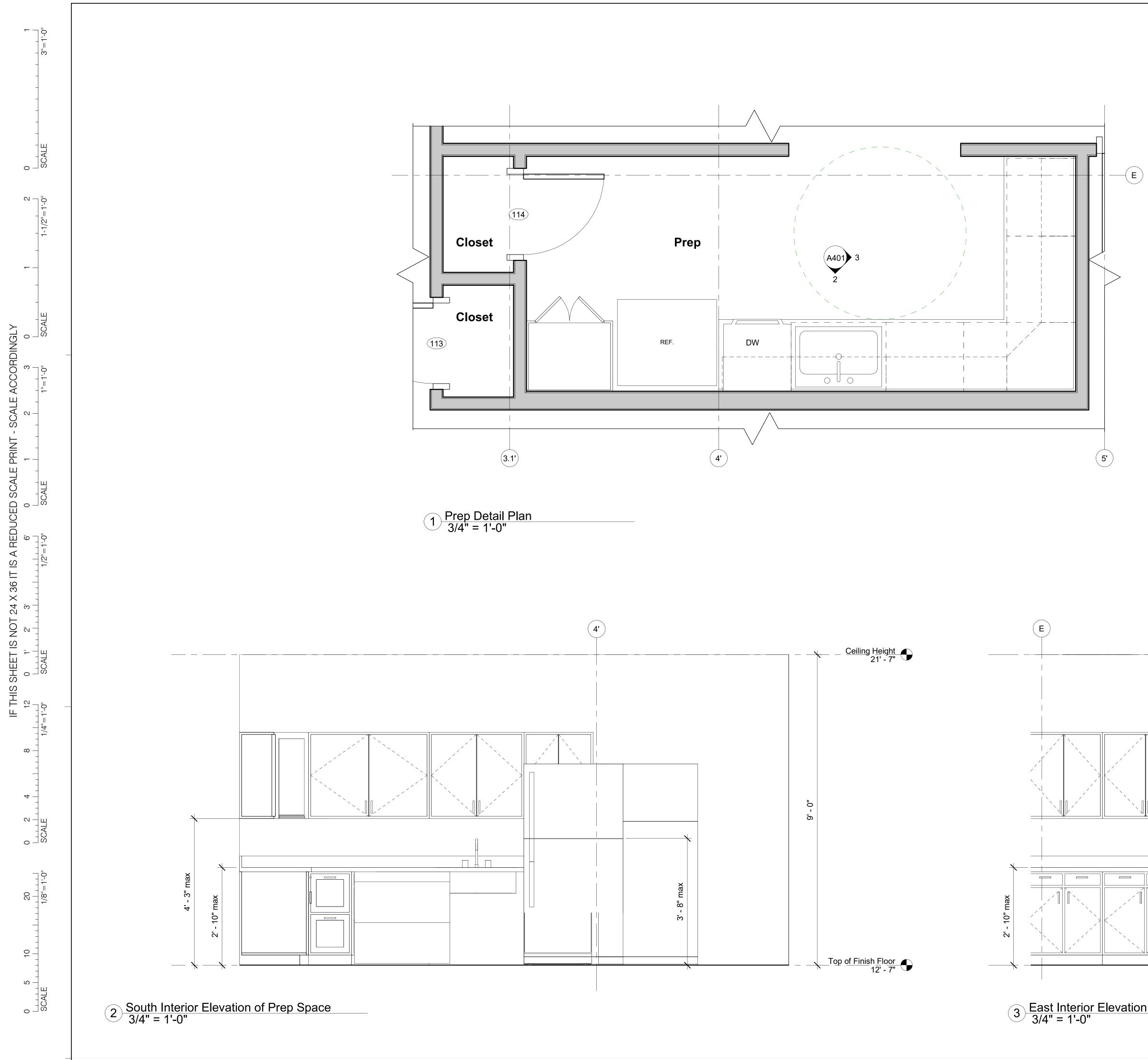
14 Salisbury Flying Horses Carousel/704.2 Salisbury Carousel House/704.2 DRAWINGS AND SPECIFICATIONS/704.2 Working Set/704.2 Flying Horses Carousel 7 Broadway 10272021.



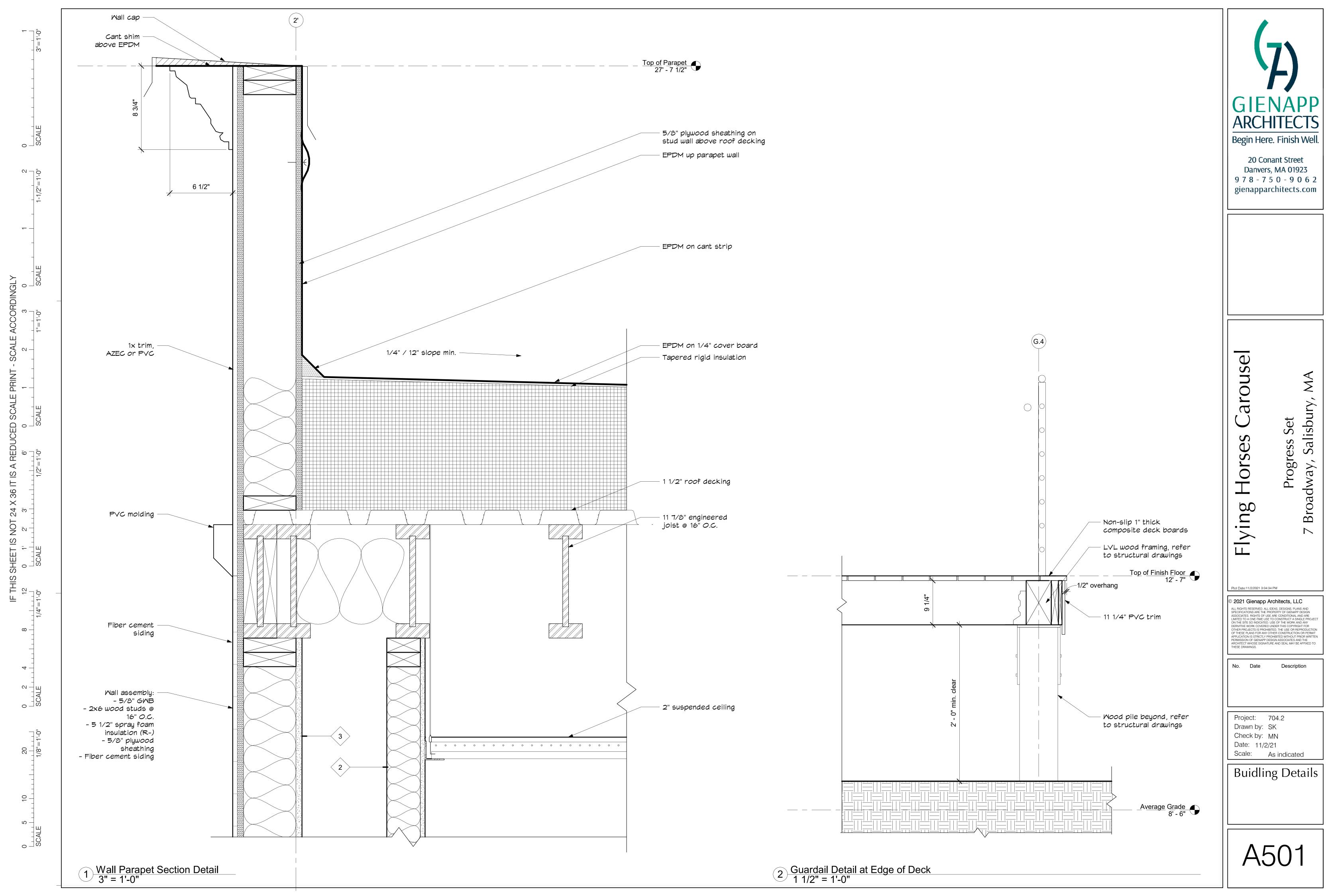


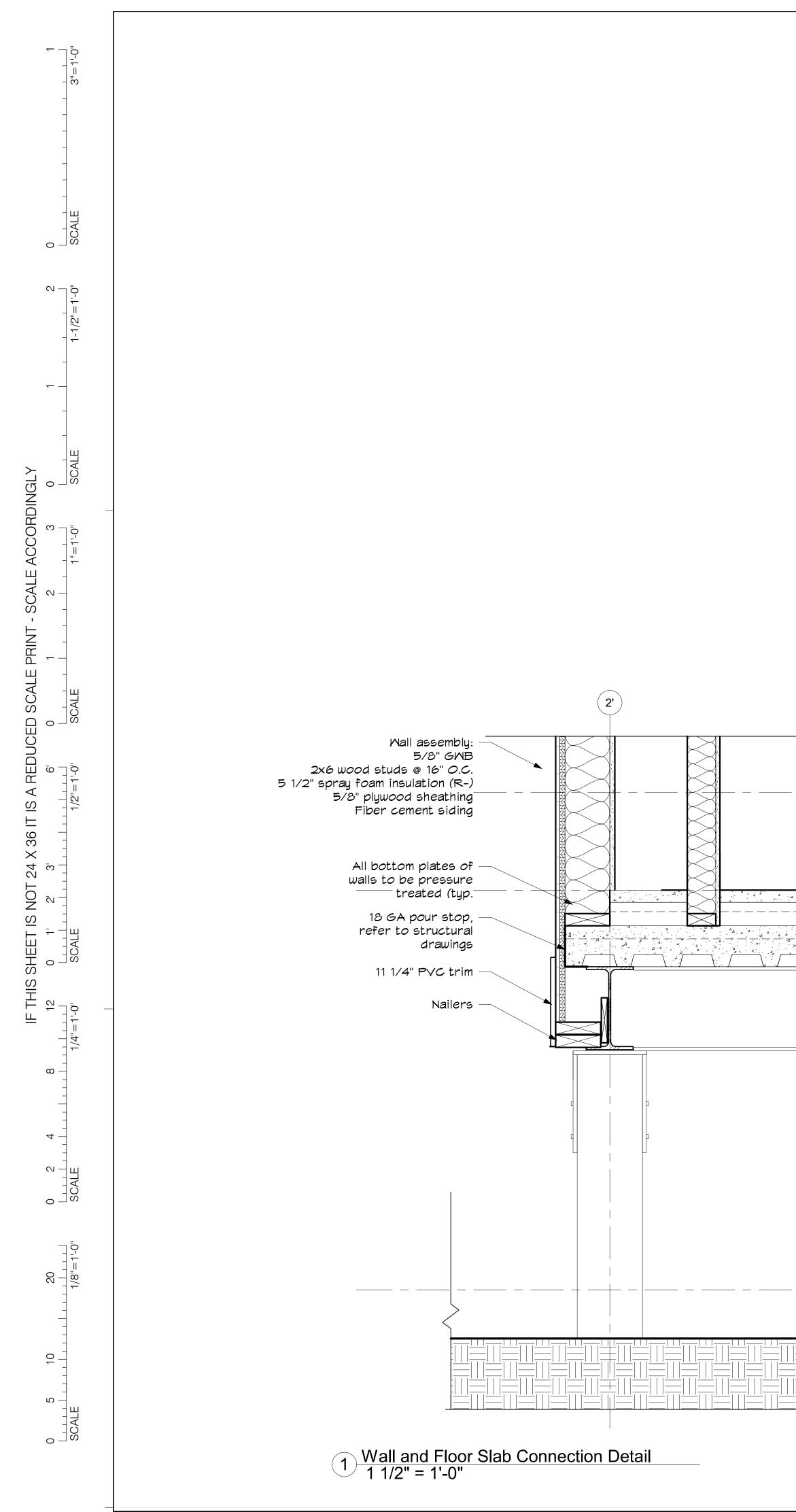
GIENAPP ARCHITECTSBegin Here. Finish Well.20 Conant Street Danvers, MA 019239 7 8 - 7 5 0 - 9 0 6 2 gienapparchitects.com
Flying Horses Carousel Progress Set 7 Broadway, Salisbury, MA
Plot Date: 11/2/2021 3:04:32 PM Image: Constraint of the property of genapp design and the property of genapp design associates. Rights of use are construct a single project of the start of a one-time use to construct a single project of the start of the property of genapp design associates and the project is producted. Use of the work and any definition of the project is producted. Use of the work and any definition of genapp design associates and the project is producted. Use of the work and any definition of genapp design associates and the project of the start of the project is producted. Use of the work and any definition of genapp design associates and the arrived of these pravision of genapp design associates and the arrived of the set of the work of the project is producted. No. Date Description Project: 704.2 Drawn by: SK Check by: MN Date: 11/2/21 Scale: 1/8" = 1'-0" North & East functioned as a start of the project of the start of
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Ceiling Height	Flying Horses Carousel	7 Broadway, Salisbury, MA
Top of Finish Floor	Plot Date: 11/2/2021 3:04:33 PM © 2021 Gienapp Architects All RIGHTS RESERVED. ALL IDEAS, DESIGN SPECIFICATIONS ARE THE PROPERTY OF GI ASSOCIATES. RIGHTS OF USE ARE CONDIT LIMITED TO A ONE-TIME USE TO CONSTRUC ON THER PROJECTS IS PROHIBITED. THE USE OTHER PROJECTS IS STRICTLY PROHIBITED WITP PRMISSION OF GIENAPP DESIGN ASSOCIA ARCHTECT WHOSE SIGNATURE AND SEAL THESE DRAWINGS. No. Date Desi Project: 704.2 Drawn by: SK Check by: MN Date: 11/2/21 Scale: 3/4" =	s, PLANS AND ENAPP DESIGN ONAL AND ARE IT A SINGLE PROJECT ORK AND ANY OR REPRODUCTION UCTION OR PERMIT HOUT PRIOR WRITTEN ITES AND THE WAY BE AFFIXED TO Scription
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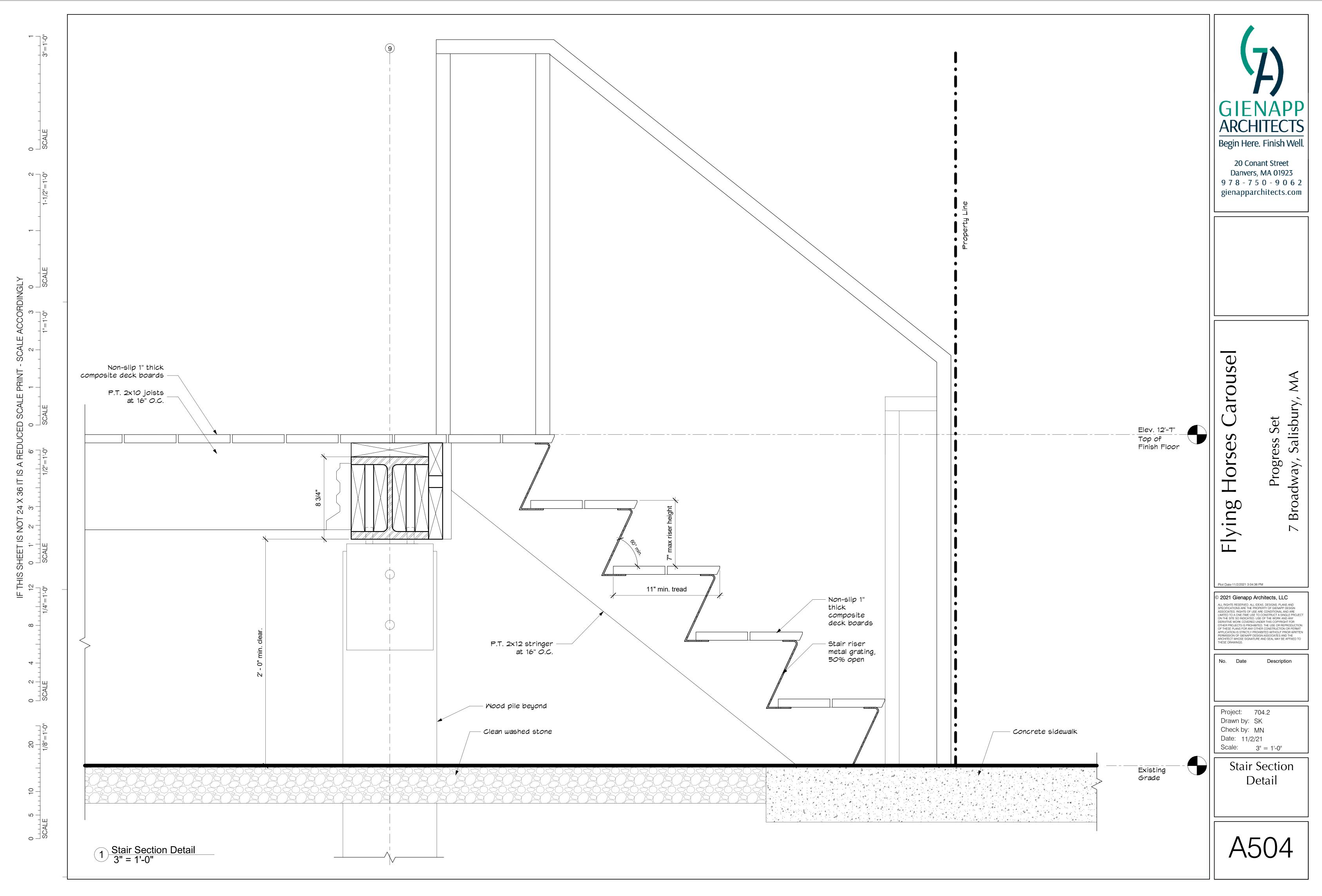




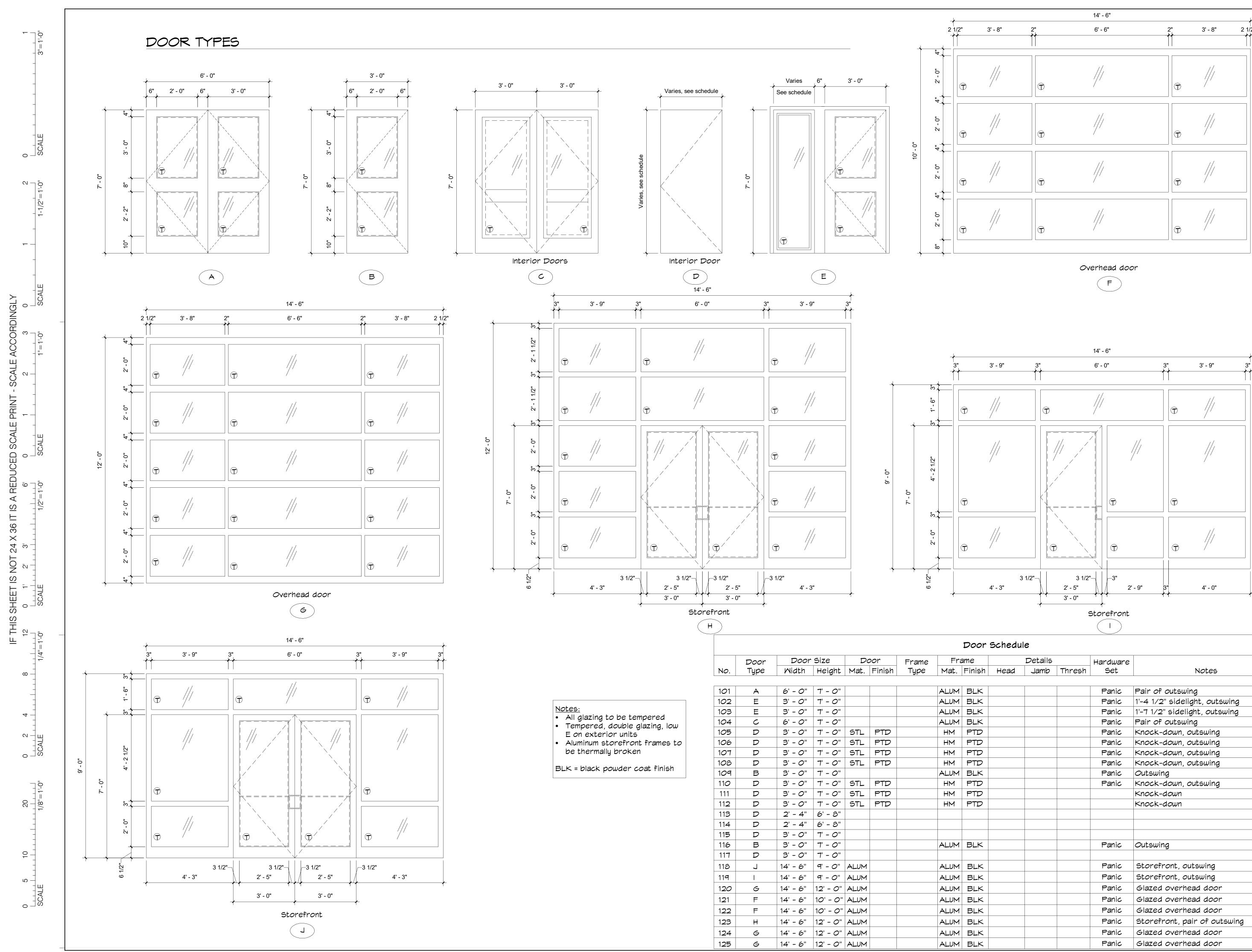
704 Salisbury Flying Horses Carousel/704.2 Salisbury Carousel House/704.2 DRAWINGS AND SPECIFICATIONS/704.2 Working Set/704.2 Flying Horses Carousel 7 Broadway 10272021.nt

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		(2) GIENAPP ARCHICES Begin Here. Finish Well 20 Conant Street Danvers, MA 01923 9 7 8 - 7 5 0 - 9 0 6 2 gienapparchitects.com
3'	Top of Carousel Finish Floor 13' - 7" Top of Finish Floor 12' - 7" 11/2" polished concrete 2 7/8" Crete Heat panel (R-10) 5" concrete slab on metal decking,	Flying Horses Carousel Progress Set 7 Broadway, Salisbury, MA
	Average Grade 8'-6"	Plot Date: 11/2/2021 3:04:35 PM Image: Construction of the second structure of the se
		A502



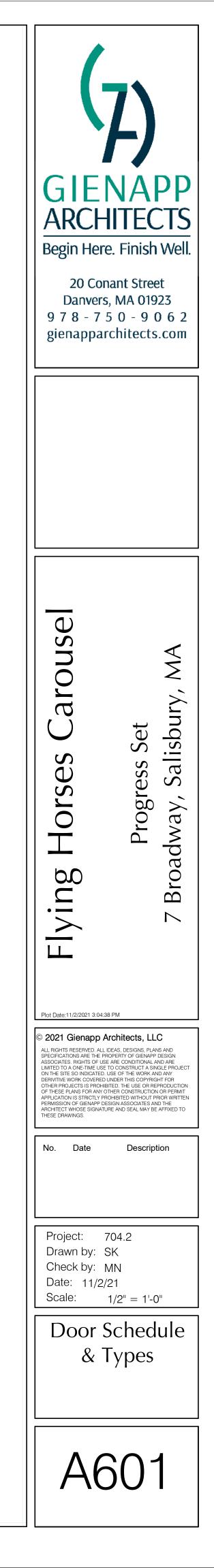
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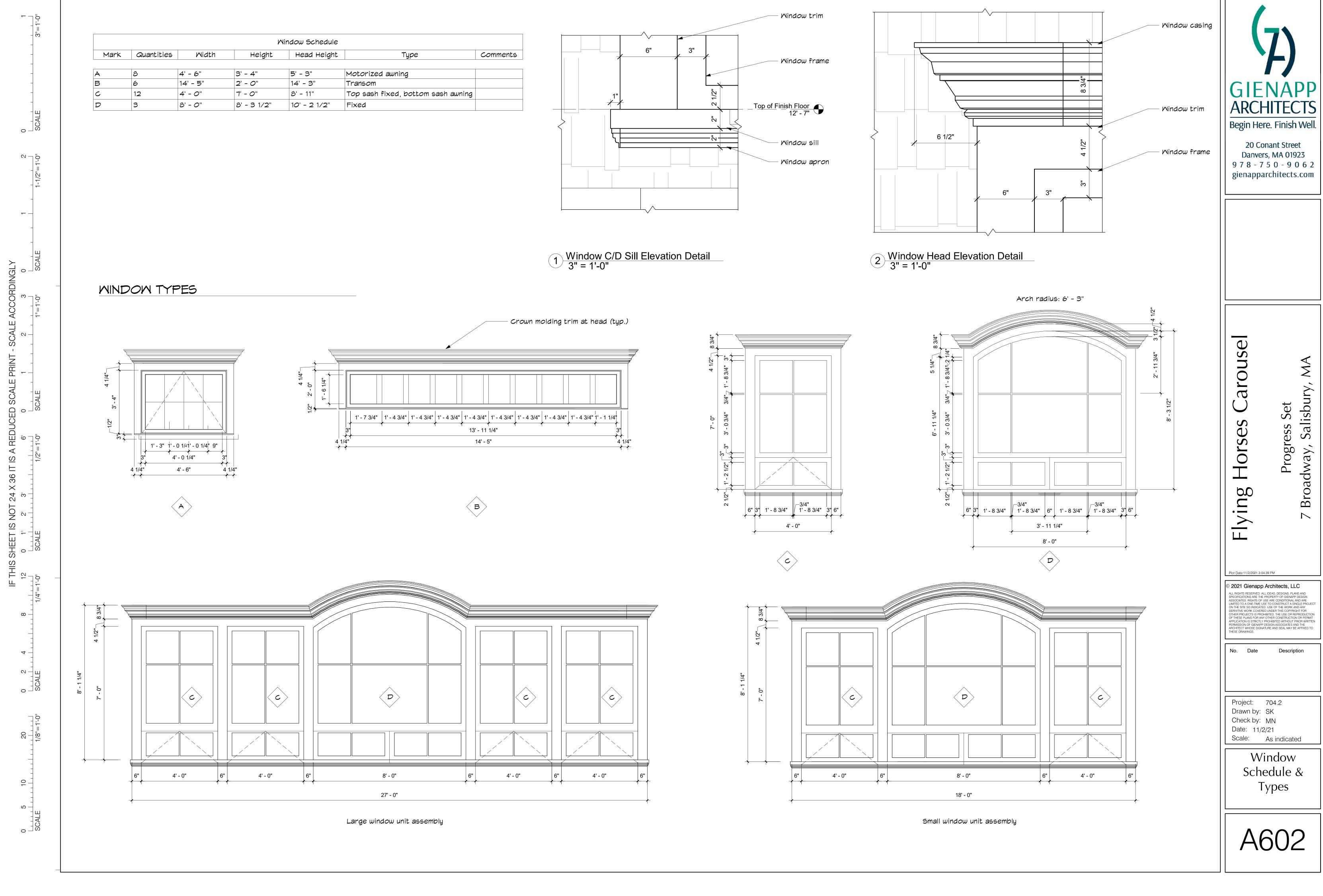


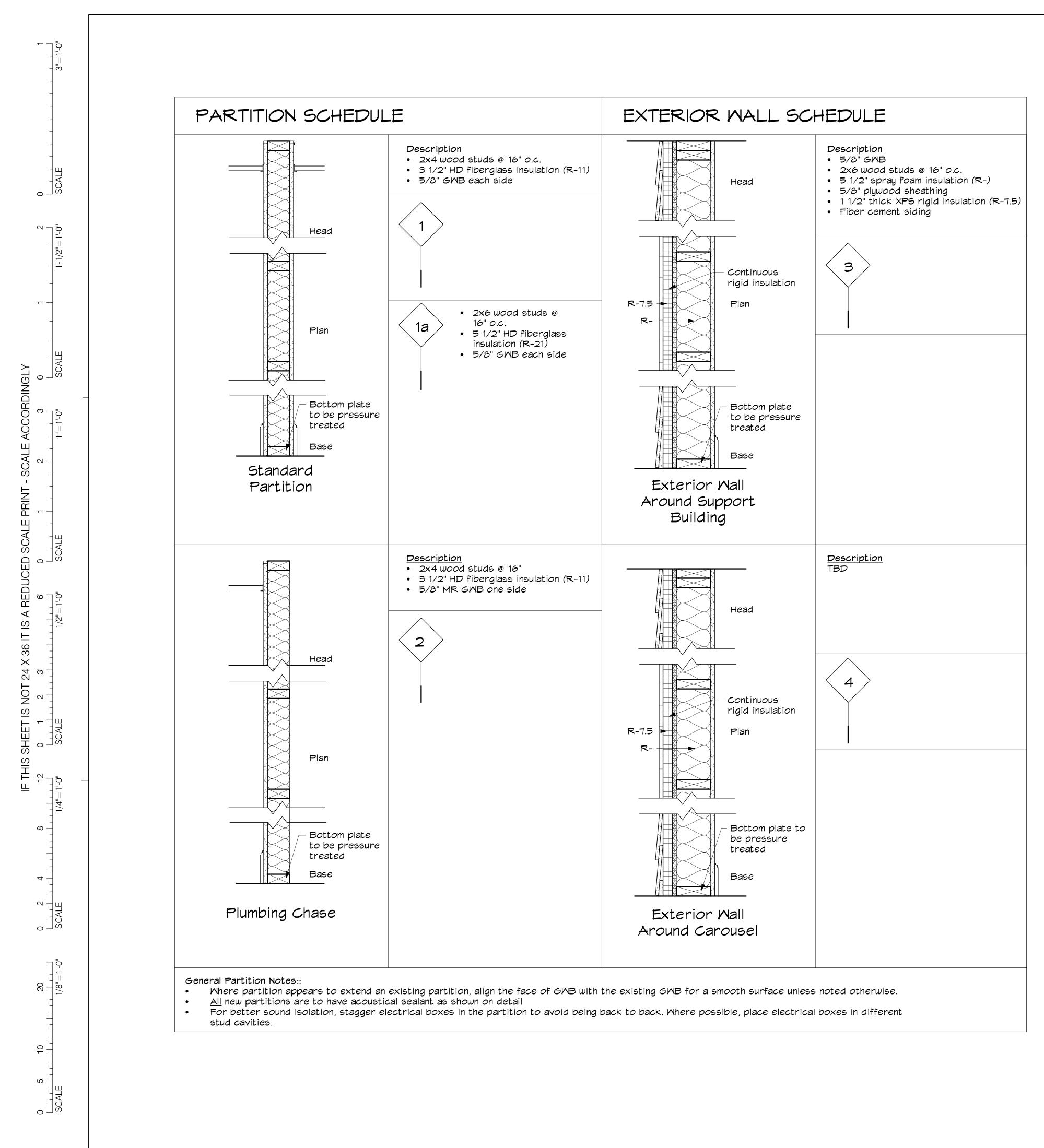
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	Door	Door			oor		ame		Details		Hardware	
NO.	Туре	Midth	Height	Mat.	Finish	Type Mat.	Finish	Head	Jamb	Thresh	Set	Notes
				1							1	1
01	A	6' - 0"	7' - 0"			ALUM	BLK				Panic	Pair of outswing
02	E	3' - 0"	7' - 0"			ALUM	BLK				Panic	1'-4 1/2" sidelight, outswing
23	E	3' - 0"	7' - 0"			ALUM	BLK				Panic	1'-7 1/2" sidelight, outswing
24	C	6' - 0"	7' - 0"			ALUM	BLK				Panic	Pair of outswing
05	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD				Panic	Knock-down, outswing
26	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD				Panic	Knock-down, outswing
70	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD				Panic	Knock-down, outswing
08	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD				Panic	Knock-down, outswing
09	В	3' - 0"	7' - 0"			ALUM	BLK				Panic	Outswing
10	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD				Panic	Knock-down, outswing
111	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD					Knock-down
12	D	3' - 0"	7' - 0"	STL	PTD	HM	PTD					Knock-down
13	D	2' - 4"	6' - 8"									
14	D	2' - 4"	6' - 8"									
15	D	3' - 0"	7' - 0"									
16	В	3' - 0"	7' - 0"			ALUM	BLK				Panic	Outswing
17	D	3' - 0"	7' - 0"									
18	L	14' - 6"	9' - 0"	ALUM		ALUM	BLK				Panic	Storefront, outswing
19	l	14' - 6"	9' - 0"	ALUM		ALUM	BLK				Panic	Storefront, outswing
20	G	14' - 6"	12' - 0"	ALUM		ALUM	BLK				Panic	Glazed overhead door
21	F	14' - 6"	10' - 0"			ALUM					Panic	Glazed overhead door
22	F	14' - 6"	10' - 0"			ALUM	BLK				Panic	Glazed overhead door
23	H	14' - 6"	12' - 0"			ALUM					Panic	Storefront, pair of outswing
24	G	14' - 6"	12' - 0"			ALUM					Panic	Glazed overhead door
25	G	14' - 6"	12' - 0"			ALUM	BLK				Panic	Glazed overhead door

		14' - 6"			<u> </u>
' - 8" 2	2"	6' - 6"	2"	3' - 8" 2 1	/2" /
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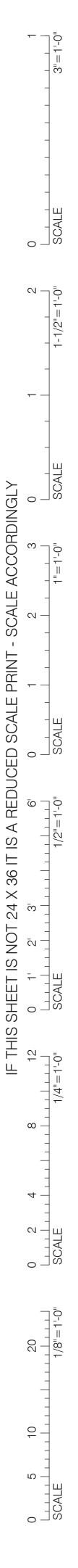






-750/704 Salisbury Flying Horses Carousel/704.2 Salisbury Carousel House/704.2 DRAWINGS AND SPECIFICATIONS/704.2 Working Set/704.2 Flying Horses Carousel 7 Broadway 10272021.nt

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Flying Horses Carouse	Progress Set 7 Broadway, Salisbury, MA
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A6	603



STRUCTURAL G	ENERAL NOTES	
A. STRUCTURAL	DESIGN CRITERIA	
1. THE STRUCTU NINTH EDITION.	IRAL DESIGN IS BASED ON THE MASSACHU	SETTS STATE BUILDING CODE, 780 CM
2. DEAD LOADS:		
	T OF MATERIALS	
M/E/P 8	MISC (TOTAL)	10 PSF
2. LIVE LOADS:		
OFFICE RETAIL	AND LOBBY	50 PSF 100 PSF
PUBLIC	STAIRS	100 PSF
LIGHT	CORRIDORS STORAGE	100 PSF 125 PSF
ROOF		20 PSF
4. SNOW LOADS	:	
BASIC (Pf (MIN	GROUND SNOW, Pg	50 PSF 30 PSF
	OOF SNOW, Pf (DESIGN) Ce	42 PSF 1.0
	Ct	1.2 1.0
DRIFT	·	AS APPLICABLE PER CODE
5. WIND LOAD -	MAIN WIND:	
BASIC	WIND SPEED (Vult)	124 MPH
lw BUILDII	NG RISK CATEGORY	1.0 II
EXPOS	URE	С
6. WIND LOAD	- COMPONENTS AND CLADDING - ROOF:	
ZONE	EFFECTIVE WIND AREA (SF)	NOMINAL PRESSURE (PSF)
1	10	44
1 1	20 50	43 41
1	100	40
2 2	10 20	76 71
2	50 100	63 56
3 3	10 20	76 71
3 3	50 100	63 56
	- COMPONENTS AND CLADDING - WALLS:	
ZONE	EFFECTIVE WIND AREA (SF)	
4	10	53
4 4	20 50	50 47
4	100	45
5	10	65
5 5	20 50	60 55
5	100	50
7. SEISMIC LOAI):	
	C IMPORTANCE FACTOR D SPECTRAL RESPONSE ACCELERATIONS:	1
	Ss S1	0.266 0.078
SITE CI		D
SFEUL	SDS	0.422
	SD1 C DESIGN CATEGORY	0.187 B
BASIC	SEISMIC RESISTING SYSTEM: - LIGHT FRAMED WALLS SHEATHED WITH FOR SHEAR RESISTANCE	WOOD STRUCTURAL PANELS RATED
	- STEEL MOMENT FRAME	
BASE S R =	HEAR	100 KIPS 3.0
Cs = EOUIVA	LENT LATERAL FORCE PROCEDURE USED	0.0938 FOR ANALYSIS
8. PHOTOVOLTA	IC PANEL SYSTEM (SOLAR PANELS) LOAD F	
DESIGN.		
B. FOUNDATION	S	
	ION IS DESIGNED AS A PILE SUPPORTED S' GEOTECHNICAL ENGINEER'S REPORT PREP.	
	1.	
BASED ON THE (DATED 09/24/202 2. CONTRACTOR	1. R IS RESPONSIBLE FOR PROPERLY BRACING RERATIONS AND DURING CONSTRUCTION.	G FOUNDATION ELEMENTS DURING

GENERAL NOTES (CONTINUED) C. REINFORCED CONCRETE SAMPLING. SAMPLES SHALL OCCUR AS FOLLOWS NOT LESS THAN (1) SET OF CYLINDERS PER DAY NOT LESS THAN (1) SET OF CYLINDERS PER EACH 150 CUBIC YARDS OF CONCRETE NOT LESS THAN (1) SET OF CYLINDERS PER 5000 SQUARE FEET OF

SURFACE AREA OF SLAB OR WALL

BE SUBMITTED TO THE ENGINEER FOR REVIEW. 3. CONTRACTOR TO PROTECT CONCRETE FROM DAMAGE DUE TO FREEZING OR HIGH TEMPERATURES. 4. CONCRETE COVER SHALL BE PER LATEST ACI REQUIREMENTS. 5. LAP BARS PER LATEST ACI REQUIREMENTS. FOR #4 BARS, LAP BARS 36 INCHES AS A MINIMUM. FOR #5 BARS, LAP BARS 48 INCHES AS A MINIMUM. FOR #6 BARS, LAP BARS 56 INCHES AS A MINIMUM. (ADJUST THESE VALUES AS REOUIRED) 6. DO NOT EXPOSE CONCRETE TO ANY CALCIUM CHLORIDES PRIOR OR DURING CONSTRUCTION. 7. IF VAPOR BARRIER IS PLACED, PLACE DIRECTLY BELOW THE SLAB ON GRADE. USE THE GREATER OF THICKNESS CITED ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS. 8. PORTLAND CEMENT MUST CONFORM TO ASTM C150 REQUIREMENTS. NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C33. NO LIGHT WEIGHT CONCRETE IS SPECIFIED FOR THE PROJECT. 9. WATER USED IN MIXES SHALL CONFORM TO ASTM C1602. 10. PROPOSED MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. MIX DESIGNS SHALL INCLUDE BUT NOT BE LIMITED TO INDICATING WATER CEMENT RATIO, AGGREGATE SIZE AND PROPORTIONING, ADMIXTURES, SLUMP, AND HISTORY OF PROPOSED MIX DESIGNS WITH SPECIFIC PROJECT EXAMPLES. 11. SUBMIT SPECIFICATIONS AND PRODUCT DATA FOR FLOOR HARDENER AND SEALER TO BE USED ON SLABS WHEN SLABS ARE TO BE REPLACED ON THE INTERIOR OF THE BUILDING. 12. SUBMIT CONCRETE REINFORCING BAR SHOP DRAWINGS TO THE ENGINEER FOR REVIEW. CONSTRUCTION OF REINFORCED CONCRETE WORK THAT PROCEEDS WITHOUT REVIEWED AND APPROVED SHOP DRAWINGS IS DONE AT THE RISK OF THE CONTRACTOR. 12. SUBMIT CONCRETE REINFORCING BAR SHOP DRAWINGS TO THE ENGINEER FOR REVIEW. CONSTRUCTION OF REINFORCED CONCRETE WORK THAT PROCEEDS WITHOUT REVIEWED AND APPROVED SHOP DRAWINGS IS DONE AT THE RISK OF THE CONTRACTOR. 13. MINIMUM MIX DESIGNS ARE AS FOLLOWS: SLAB ON GRADE AND ELEVATED SLAB: MAXIMUM W/C RATIO 0.5 AIR ENTRAINMENT N/A CEMENT TYPE II 3,500 PSI MAXIMUM AGGREGATE 3/4" 14. AS A MINIMUM REINFORCED ALL ELEVATED SLAB WITH 6X6 - W1.4XW1.4 WELDED WIRE MESH. USE HEAVIER REINFORCING IF NOTED ON THE PLANS. LAP WELDED WIRE A MINIMUM OF 12 INCHES. E. WOOD 1. WOOD MEMBERS SHALL BE AS PER THE DRAWINGS. MEMBERS OF EQUIVALENT STRENGTH AND STIFFNESS MAY BE SUBSTITUTED IF PERMITTED BY THE ARCHITECT/ENGINEER. USE SPRUCE PINE FIR No.2 AS A MINIMUM. ALL PRESSURE TREATED LUMBER SHOULD BE SOUTHERN PINE #1. 2. WALLS INTERIOR BEARING WALL: UNLESS INDICATED OTHERWISE ON THE DRAWINGS, USE THE FOLLOWING: 2x4 @ 16" O.C. SPRUCE PINE FIR No.2 OR 2x6 @ 16" O.C. SPRUCE PINE FIR No.2 EXTERIOR WALLS: UNLESS INDICATED OTHERWISE ON THE DRAWINGS, USE THE FOLLOWING: 2x6 @ 16" O.C. SPRUCE PINE FIR No.2 INTERIOR NON-BEARING WALLS: UNLESS INDICATED OTHERWISE ON THE DRAWINGS, USE THE FOLLOWING: 2x4 @ 16" O.C. SPF. No.2 3. PLYWOOD AND OTHER SIMILAR SHEATHING MATERIALS SHALL BE AS PER THE DRAWINGS. APA RATED MATERIALS SHALL BE USED. THE STRONG AXIS OF SHEATHING MATERIALS SHALL RUN PERPENDICULAR TO THE FLOOR FRAMING AND WALL STUDS 4. HANGERS, CLIPS, ETC SHALL BE AS PER THE DRAWINGS. CONTRACTOR TO BRING ANY UNIDENTIFIED HANGARS, ETC TO THE ATTENTION OF THE ENGINEER FOR RESOLUTION. 5. EXTERIOR WALL SHEATHING SHALL BE A MINIMUM IF ½ INCH APA RATED, EXPOSURE 1 SHEATHING NAILED TO THE WALL FRAMING W/ 8d NAILS AT 6 INCH CENTERS AT PANEL EDGES AND 12 INCH CENTERS AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE.

6. INTERIOR WOOD SHEATHING USED AS SHEAR WALL SHEATHING SHALL BE A MINIMUM IF ½ INCH, APA RATED SHEATHING NAILED PER THE SHEAR WALL DRAWINGS AND SCHEDULES. 7. FLOOR AND ROOF SHEATHING SHALL BE PER THE FLOOR AND ROOF FRAMING DRAWINGS.

8. ALL ENGINEERED LUMBER SHALL BE AS PER THE DRAWINGS. ALL LSL MEMBERS SHALL HAVE A MINIMUM YOUNG'S MODULUS (E) OF 1,550,000 PSI; ALL LVL'S SHALL HAVE A MINIMUM MODULUS (E) OF 2,000,000 PSI; ALL PSL'S USED AS BEAMS SHALL HAVE A MINIMUM YOUNG'S MODULUS (E) OF 2,000,000 PSI; ALL PSL'S USED AS POSTS OR COLUMNS SHALL HAVE A MINIMUM YOUNG'S MODULUS (E) OF 1,800,000 PSI. ENGINEERED LUMBER MUST HAVE THE IDENTIFICATION MARKINGS LEFT ON FOR FIELD VERIFICATION PURPOSES.

9. ALL ENGINEERED WOOD "I" JOISTS SHALL BE AS PER THE DRAWINGS. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.

10. USE ENGINEERED LUMBER MANUFACTURERS GUIDELINES FOR MEMBER WEB OR FLANGE PENETRATIONS, MAXIMUM ALLOWED NOTCHES, BRIDGING REQUIREMENTS, INTERIOR AND EXTERIOR BEARING REQUIREMENTS, ETC.

11. CONTACT THE ENGINEER REGARDING ALL DIMENSIONAL LUMBER PENETRATIONS, NOTCHES, ETC.

12. AT OPENINGS LARGER THAN 4 FEET IN FLOOR SHEATHING, PROVIDE BLOCKING BEYOND HEADERS AS WELL AS 1-1/2" 16GA STRAPS WITH (8) 16d NAILS ON EACH SIDE OF THE HEADER-JOIST INTERSECTION. THE METAL STRAPS SHALL BE OF 33,000 PSI OR GREATER. SEE FIGURE 2308.11.3.3 IN THE INTERNATIONAL BUILDING CODE 2015.

F. TIMBER PILES

1. THE TIMBER PILE FOUNDATION SOLUTION SHOWN ON THE DRAWINGS IS BASED ON THE RECOMMENDATIONS NOTED IN THE MCPHAIL ASSOCIATES, LLC. GEOTECHNICAL REPORT DATED SEPTEMBER 24, 2021.

2. BASED ON INFORMATION FOUND IN THE CITED GEOTECHNICAL REPORT, ALL TIMBER PILES SHALL HAVE A MINIMUM COMPRESSIVE CAPACITY OF 20 TONS (40,000 POUNDS). THIS VALUE ASSUMES A FACTOR OF SAFETY (FS) VALUE OF 2.5. EACH PILE SHALL HAVE A MINIMUM LATERAL CAPACITY OF 1,800 POUNDS FOR A MAXIMUM OF 1 INCH HORIZONTAL PILE MOVEMENT

3. BASED ON INFORMATION FOUND IN THE CITED GEOTECHNICAL REPORT, THE MINIMUM TIMBER PILE TIP DIAMETER SHOULD BE 8 INCHES. THE PILE DRIVING HAMMER SHOULD HAVE A MINIMUM ENERGY OF 9,000 FOOT-POUNDS PER BLOW AND MAXIMUM ENERGY OF 12,000 FOOT-POUNDS PER BLOW. EACH PILE SHOULD BE SPACED AT LEAST 30 INCHES ON CENTER.

4. BASED ON INFORMATION FOUND IN THE CITED GEOTECHNICAL REPORT, THE END BEARING OF THE PILES SHALL BE IN THE NATURAL SAND DEPOSIT AND OR ON BEDROCK. EACH PILE SHALL HAVE A MINIMUM EMBEDMENT OF 37 FEET BELOW THE GROUND SURFACE AND A MAXIMUM PROJECTION OF 3-FOOT ABOVE GROUND SURFACE.

5.BASED ON INFORMATION FOUND IN THE CITED GEOTECHNICAL REPORT, THE PILES ARE RECOMMENDED TO BE PRESSURE TREATED WITH CHROMATED COPPER ARSENATE (CCA) PRESERVATIVE IN ACCORDANCE WITH STANDARD C3 OF THE AMERICAN WOOD PRESERVERS ASSOCIATION.

6. PILES SHALL NOT BE RELOCATED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER. 7. A DAILY PHOTOGRAPHIC JOURNAL SHALL BE MAINTAINED. THE PHOTOGRAPHS SHOULD CAPTURE WORK IN PROGRESS. PHOTOGRAPHS SHALL INCLUDE, BUT SHALL NOT BE LIMITED TO IN PROGRESS PILE INSTALLATION, PILE BRACKET INSTALLATION, AND PILE RELOCATIONS.

8. A PILE DRIVING LOG SHALL BE KEPT ON SITE AT ALL TIMES AND SHALL BE MAINTAINED AND UPDATED DAILY. THE LOG SHALL DOCUMENT ALL PILE DRIVING ACTIVITIES.

9. AN "AS-BUILT" DRAWING SHALL BE MAINTAINED BY THE PILE DRIVING CONTRACTOR. A FINAL COPY OF THE "AS-BUILT" DRAWING SHALL BE SUBMITTED TO THE GENERAL CONTRACTOR, THE GEOTECHNICAL ENGINEER, THE OWNER, THE ARCHITECT, AND THE STRUCTURAL ENGINEER.

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AS NOTED IN THE MINIMUM MIX DESIGNS. THE NOTED STRENGTHS ARE AT 28 DAYS. AIR ENTRAINMENT SHALL BE AS NOTED IN THE MINIMUM MIX DESIGNS. SUBMIT CONCRETE BREAK TEST REPORTS TO THE ENGINEER FOR REVIEW. THREE TEST CYLINDERS SHALL BE TAKEN PER

RESULTS SHALL BE SUBMITTED FOR BREAKS OCCURRING AT 7, 28, AND 56 DAYS.

2. ALL REINFORCING BARS TO BE ASTM A615. ALL WELDED WIRE TO BE ASTM A185. MATERIAL CERTIFICATIONS MUST



G. STRUCTURAL STEEL J. SUBMITTALS 1. ALL WIDE FLANGE SECTIONS SHALL BE FABRICATED FROM ASTM A992 STRUCTURAL STEEL; ALL CHANNELS ANGLES, AND PLATES SHALL BE FABRICATED FROM ASTM A36 STRUCTURAL STEEL; ALL HSS SECTIONS SHALL BE FABRICATED FROM ASTM A500 GRADE B STRUCTURAL STEEL WITH A MINIMUM YIELD STRESS OF 50 KSI; ALL PIPE SECTIONS SHALL BE FABRICATED FROM ASTM A53, GRADE B STRUCTURAL STEEL WITH A MINIMUM YIELD STRESS OF 35 KSI. 2. ALL CONNECTIONS SHALL BE PER THE LATEST EDITION OF THE AISC STEEL CONSTRUCTION MANUAL. 3. ALL BOLTS SHALL BE MIN 3/4 INCH DIAMETER ASTM A325 TYPE N. 4. ALL WELDS SHALL BE WITH E70XX ELECTRODES; WELDING SHALL CONFORM TO THE LATEST AWS D1.1. 5. BELOW ALL BASE PLATES PROVIDE 1/4 INCH LEVELING PLATE AND 3/4" NON-SHRINK GROUT, UNLESS SHOWN GENERAL: OTHERWISE ON THE DRAWINGS. UNLESS OTHERWISE NOTED, ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554, AND SHALL BE GRADE 36. ASSOCIATED THE SPECIAL INSPECTIONS. USE 3/4 INCH DIAMETER RODS WITH NINE INCHES OF EMBEDMENT AND 4 INCHES OF PROJECTION, UNLESS OTHERWISE NOTED (UON) ON THE DRAWINGS. 7. TEMPORARY BRACING, SHORING, ETC, IS THE RESPONSIBILITY OF THE CONTRACTOR. 8. IF CLASHES OCCUR BETWEEN STRUCTURAL STEEL AND OTHER TRADES, CONTACT THE ARCHITECT TO RESOLVE THE CLASHES. FIELD CUTTING OF MEMBERS WILL NOT BE PERMITTED. 9. SUBMIT STRUCTURAL STEEL SHOP DRAWINGS FOR REVIEW TO THE ENGINEER. SHOP DRAWINGS AND <u>soils</u> INFORMATION INCLUDED ON SHOP DRAWINGS SHALL INCLUDE BUT NOT BE LIMITED TO ERECTION PLANS, PIECE DRAWINGS, MATERIAL SPECIFICATION, COATING SPECIFICATION, FASTENER DATA, AND METHOD OF SURFACE PREPARATION TO RECEIVE PAINT. 10. AS A MINIMUM, STRUCTURAL STEEL SHALL BE HOT DIP GALVANIZED IN AREAS WHERE STRUCTURAL STEEL ELEMENTS ARE EXPOSED TO THE ELEMENTS. IN ADDITION, ALL HARDWARE (BOLTS, NUTS, WASHERS) SHALL BE HOT DIP GALVANIZED.

11. ALL CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR. CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD(E.O.R.) FOR REVIEW. THE CALCULATIONS SHALL BEAR THE SIGNATURE AND SEAL OF A MASSACHUSETTS REGISTERED STRUCTURAL ENGINEER.

12. LOADS GIVEN ON THE DRAWINGS ARE IN ALLOWABLE STRESS FORMAT.

H. METAL FLOOR DECK

1. ALL METAL DECK SHALL BE BY VULCRAFT OR APPROVED EQUIVALENT

2. ALL METAL DECK SHALL BE AS FOLLOWS:

FLOOR DECK: VULCRAFT 2C 20 GA STEEL DECK FLOOR DECK SHALL BE NON COMPOSITE

- DECK THICKNESS = 0.0358 INCHES
- MOMENT OF INERTIA $(+) = 0.409 \text{ IN}^4$ MOMENT OF INERTIA $(-) = 0.406 \text{ IN}^4$
- POSITIVE SECTION MODULUS (Sp) = 0.341 INCHES³ PER FOOT
- NEGATIVE SECTION MODULUS (Sn) = 0.346 INCHES³ PER FOOT YIELD STRENGTH = 50.000 PSI
- REINFORCING IF NOT INDICATED ON THE DRAWINGS = #6 BARS @ 12" O.C. EACH WAY. PLACE
- REINFORCEMENT 4" BELOW TOP OF CONCRETE SLAB. TOTAL SLAB DEPTH IF NOT INDICATED ON THE DRAWINGS = 8 INCHES

3. ALL DECK SHALL BE INSTALLED IN A MULTI SPAN FASHION WHERE POSSIBLE. CONNECTIONS OF DECK SHALL BE AS SHOWN ON THE DRAWINGS. IF DECK CONNECTIONS ARE NOT SHOWN CONTACT THE ENGINEER.

4. IF NOT INDICATED ON THE DRAWINGS, ALL DECK SHALL BE FASTENED TO THE SUPPORT MEMBERS BY WELDING. ALL WELDS SHALL BE WITH E70XX ELECTRODES; WELDING SHALL CONFORM TO THE LATEST AWS D1.1. SCREW FASTENERS AND POWDER ACTUATED FASTENERS ARE PERMITTED BUT DESIGN DATA FOR ALTERNATE FASTENING METHODS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW.

5. ALL DECK SECTION PROPERTIES SHALL CONFORM TO AISI AND SDI REQUIREMENTS AND STANDARDS.

6. ALL FLOOR DECK SHALL BE GALVANIZED TO ACHIEVE A G-90 COATING.

7. INSTALL ALL DECK PER AISI AND SDI RECOMMENDATIONS.

8. STORE ALL DECK MATERIALS OFF THE GROUND AND IN A MANNER THAT ALLOWS ANY WATER TO DRAIN UNDER THE STORED MATERIAL. DECK SHALL BE COVERED UNTIL USE.

9. SUBMIT FLOOR DECK SHOP DRAWINGS FOR REVIEW TO THE ENGINEER. SHOP DRAWINGS AND INFORMATION INCLUDED ON SHOP DRAWINGS SHALL INCLUDE BUT NOT BE LIMITED TO ERECTION PLANS, PIECE MARKINGS, MATERIAL SPECIFICATION, COATING SPECIFICATION, FASTENER DATA, AND FASTENING PATTERNS.

I. CONTRACTOR'S RESPONSIBILITIES

1. CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY SHORING AND TEMPORARY BRACING DESIGN RELATED TO MEANS AND METHODS OF CONSTRUCTION.

2. COORDINATION BETWEEN TRADES IS THE RESPONSIBILITY OF THE CONTRACTOR. DRAWING CONFLICTS SHOULD BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND ARCHITECT FOR RESOLUTION.

3. THE CONTRACTOR IS REQUIRED TO INFORM THE DESIGN PROFESSIONAL OF THE PROGRESS OF THE PROJECT IN ORDER FOR THE DESIGN PROFESSIONAL TO PERFORM PERIODIC SITE VISITS TO OBSERVE THE WORK IN PROGRESS.

4. SECTIONS AND DETAILS DEPICT SPECIFIC CONDITIONS. ALL CONDITIONS ARE NOT SHOWN ON THESE DRAWINGS. CONDITIONS SIMILAR TO THOSE SHOWN ON THE DRAWINGS SHALL UTILIZE SIMILAR DETAILS.

5. THE DRAWINGS ARE NOT TO BE SCALED. THE CONTRACTOR IS TO CONTACT THE DESIGN PROFESSIONAL REGARDING ANY DIMENSIONAL INQUIRIES.

REQUIREMENTS. DEEP FOUNDATIONS IBC TABLE 1705.7.

> 2. FULL TIME OBSERVATIONS OF PILE LOAD TEST AND PILE INSTALLATION OPERATIONS WILL BE PERFORMED BY THE GEOTECHNICAL ENGINEER. PILES SHALL ONLY BE INSTALLED IN THE PRESENCE OF THE GEOTECHNICAL ENGINEER.

<u>CONCRETE</u>

DRAWING REQUIREMENTS.

2. THE CONCRETE INSPECTIONS AND VERIFICATIONS MUST COMPLY WITH TABLE 1705.3 OF THE 2015 INTERNATIONAL BUILDING CODE (IBC).

WOOD AND ENGINEERED WOOD PRODUCTS

1. SUPPLIER OF ENGINEERED WOOD PRODUCTS (LVL'S, WOOD 'I' JOISTS, GLU-LAMS, PRE ENGINEERED FLOOR AND ROOF TRUSSES, AND PLYWOOD) SHALL OBTAIN AND SUBMIT TO THE ENGINEER THE FABRICATOR'S WRITTEN PROCEDURE FOR QUALITY CONTROL AND QUALITY ASSURANCE FOR ENGINEERED WOOD PRODUCTS. PRODUCTS SHALL BE SUBMITTED WITH A CERTIFICATE OF COMPLIANCE TO THE ENGINEER STATING THAT THE PRODUCTS WERE SUPPLIED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND WERE FABRICATED IN ACCORDANCE WITH THE QUALITY ASSURANCE QUALITY CONTROL PROCEDURES.

2. PERIODIC SPECIAL INSPECTIONS ARE REQUIRED FOR ALL WOOD FRAMING OPERATIONS INCLUDING BUT NOT LIMITED TO WALL CONSTRUCTION, FLOOR CONSTRUCTION, SHEAR WALL CONSTRUCTION, NAILING, BOLTING, FASTENING, HURRICANE TIE INSTALLATION, LATERAL TIE INSTALLATION, AND HANGER INSTALLATION.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE TO THE REQUIREMENTS OF IBC SECTION 1705.2.1 AND AISC 360.

CODE, 780 CMR, 9TH EDITION.

METAL FLOOR DECK

1. SUBMIT SHOP DRAWINGS FOR THE FOLLOWING: STRUCTURAL STEEL

CONCRETE REINFORCING WOOD FRAMING AS INDICATED IN NOTES

METAL DECK PILE DATA REQUIRED BY SECTION F

PILE GRADE AND PRESERVATIVE MATERIALS SUBMIT PROPOSED MIX DESIGNS FOR CONCRETE. SUBMIT ALL ASSOCIATED ADMIXTURES INCLUDING BUT NOT LIMITED TO WATER REDUCERS, PLASTICIZERS, AND AIR ENTRAINMENT.

SPECIAL INSPECTIONS AND FABRICATOR/ CONTRACTOR DOCUMENTATION REQUIREMENTS:

1. THE OWNER IS RESPONSIBLE FOR PAYMENT TO THE INSPECTION AGENCIE(S) THAT PERFORM THE WORK

1. THE OWNER MAY UTILIZE THE SERVICES OF THE GENERAL CONTRACTOR FOR THE PURPOSE OF HIRING AND SUPERVISING THE SPECIAL INSPECTORS.

2. ALL SPECIAL INSPECTION AGENCIES THAT ARE BEING CONSIDERED TO PERFORM THE WORK SHALL SUBMIT A QUALIFICATIONS STATEMENT TO THE GENERAL CONTRACTOR FOR SUBMITTAL TO THE ENGINEER. THE ENGINEER SHALL REVIEW THE QUALIFICATIONS AND MAKE RECOMMENDATIONS TO THE GENERAL CONTRACTOR.

1. IT IS ANTICIPATED SOIL INSPECTIONS WILL BE REQUIRED. THE OWNER WILL BE RESPONSIBLE FOR HIRING A GEOTECHNICAL DESIGN PROFESSIONAL AND A SOIL INDEPENDENT INSPECTION AGENCY. THE GEOTECHNICAL ENGINEER WILL COORDINATE WITH THE INDEPENDENT INSPECTION AGENCY TO PERFORM SPECIAL INSPECTIONS. IF SPECIAL INSPECTIONS ARE REQUIRED SEE BELOW FOR INSPECTION AGENCY REQUIREMENTS.

SOILS INSPECTION AGENCY MINIMUM REQUIREMENTS:

1. INSPECTION AGENCY SHALL REVIEW THE EXCAVATION CONTRACTOR'S QUALITY CONTROL PROCEDURES PRIOR TO COMMENCEMENT OF EXCAVATION WORK. REVIEW FOR ADHERENCE TO THE PROCEDURES SHALL OCCUR ON A WEEKLY BASIS.

2. INSPECTION AGENCY SHALL INSPECT EXCAVATIONS PRIOR TO THE START OF FOUNDATION, WORK. INSPECTION SHALL INCLUDE VERIFICATION OF CORRECT EXCAVATION DEPTH VERIFICATION OF SUITABILITY OF BEARING STRATA RELEVANT TO THE REQUIREMENTS SET FORTH ON THE DRAWINGS, AND VERIFICATION THAT EXCAVATION OPERATIONS ARE CONTROLLED AND NO UNSUITABLE MATERIALS ARE CONTAINED WITHIN THE EXCAVATION.

3. STRUCTURAL FILL OPERATIONS SHALL BE MONITORED BY THE INSPECTION AGENCY. AGENCY SHALL REVIEW CONTRACTORS QUALITY CONTROL PROCEDURES. AGENCY SHALL VERIFY THAT FILL MATERIALS ARE OF SUITABLE MATERIAL AND THAT INSTALLATION CONFORMS TO THE PROJECT REQUIREMENTS RELATIVE TO OPTIMUM WATER CONTENT AND IN PLACE DENSITY

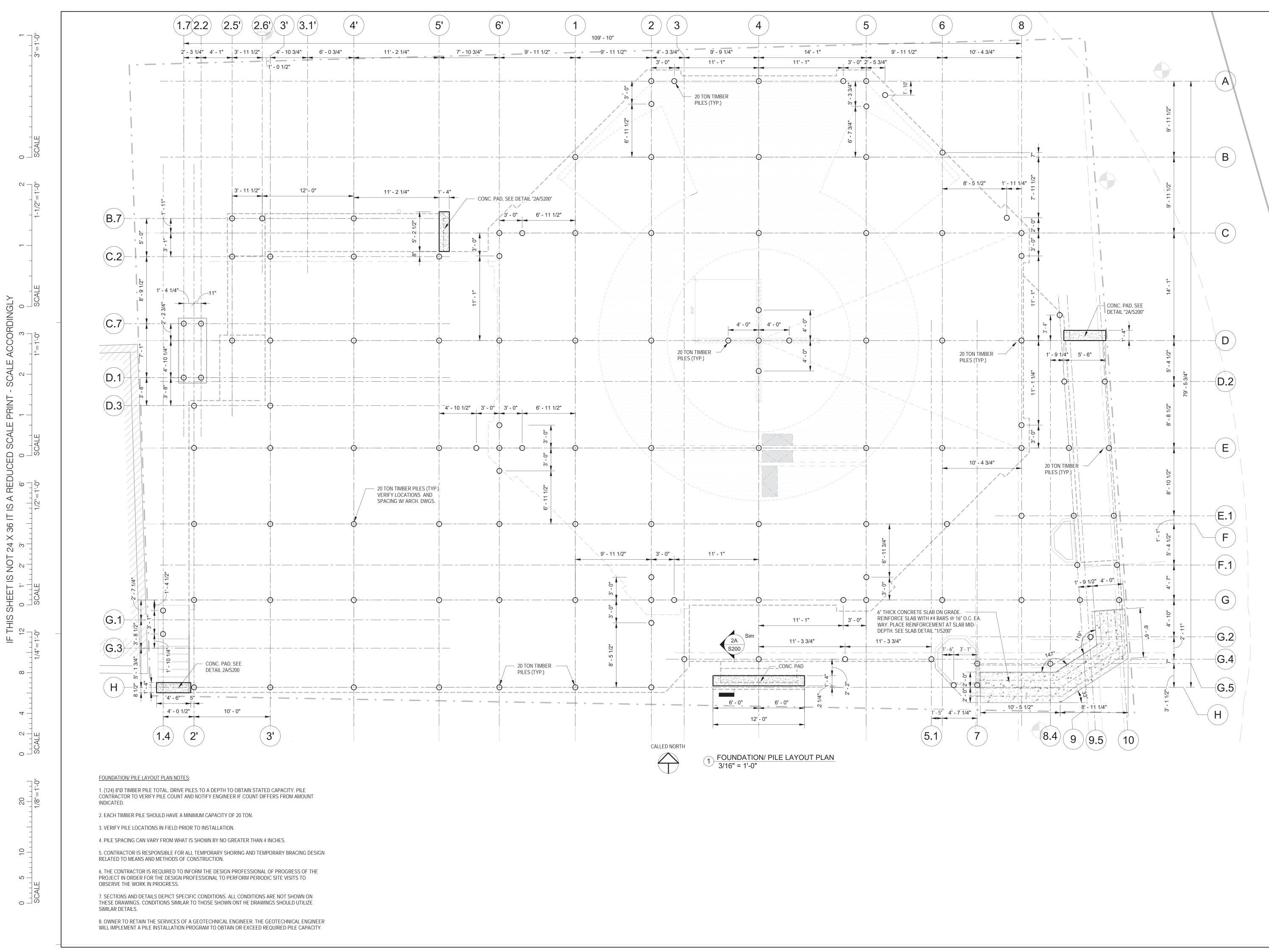
1. DRIVEN PILES SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE TO THE REQUIREMENTS OF 2015

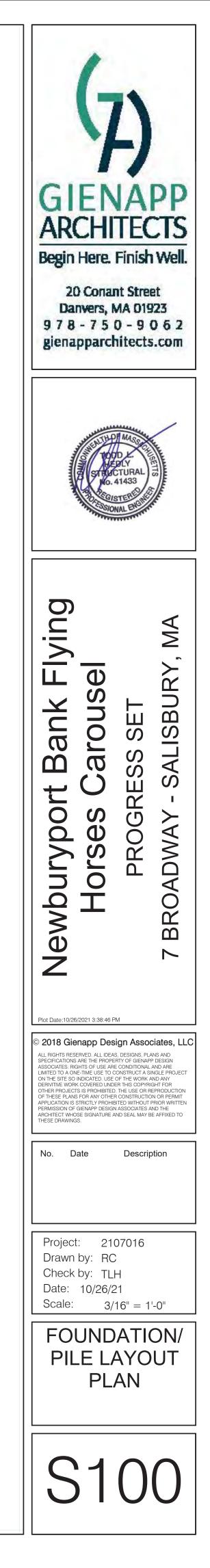
1. IT IS ANTICIPATED CONCRETE INSPECTIONS WILL BE REQUIRED. THE ENGINEER OF RECORD WILL REVIEW PROPOSED MIX DESIGNS. THE GENERAL CONTRACTOR (BY MEANS OF THE INDEPENDENT TESTING AGENCY) MUST INSURE CONCRETE PLACED ON SITE MATCHES THE REVIEWED CONCRETE MIX DESIGN. ALSO, THE CONTRACTOR (BY MEANS OF THE INDEPENDENT TESTING AGENCY) MUST ENSURE REINFORCING PLACED MATCHES WHAT HAS BEEN REVIEWED BY THE ENGINEER OF RECORD ON THE SHOP DRAWINGS. THE CONTRACTOR (AND THE INDEPENDENT TESTING AGENCY) MUST OBSERVE CONCRETE PLACEMENT, CURING, AND PROTECTION PROCEDURES DURING CONSTRUCTION AND ENSURE CONFORMANCE TO ACI STANDARDS AND

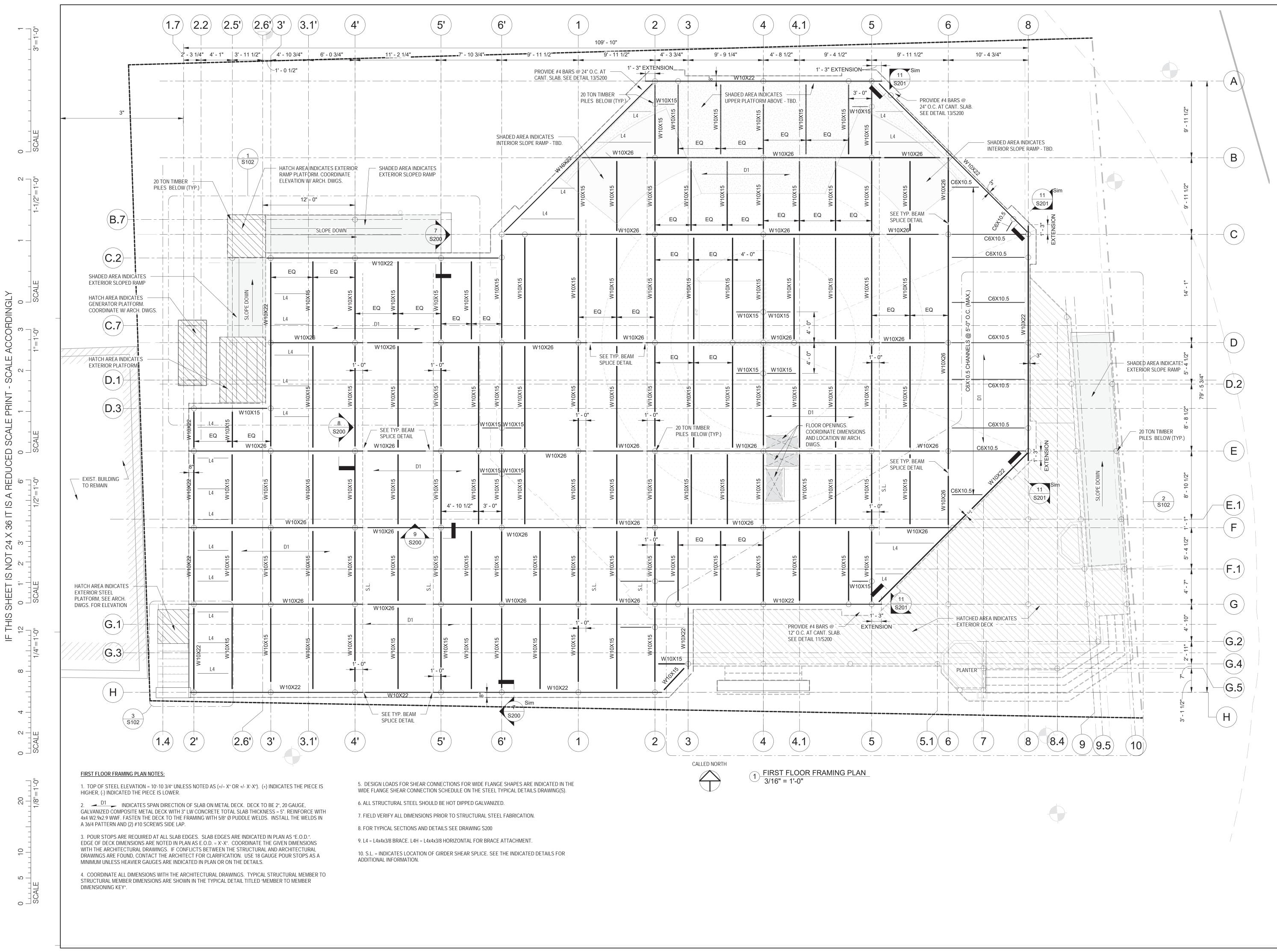
2. SPECIAL INSPECTIONS REQUIREMENTS FOR THE STRUCTURAL STEEL FABRICATOR (IN ACCORDANCE WITH IBC SECTION 1705.2) SHALL BE WAIVED IF THE FABRICATOR MEETS THE EXCEPTIONS OF SECTION 1705.2 IN ACCORDANCE WITH THE MASSACHUSETTS STATE BUILDING

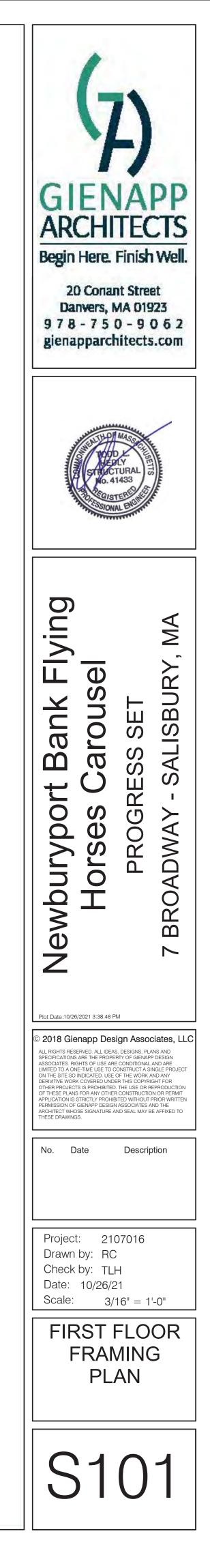
1. CONNECTIONS OF DECK TO FRAMING SHALL BE CONDUCTED PERIODICALLY.

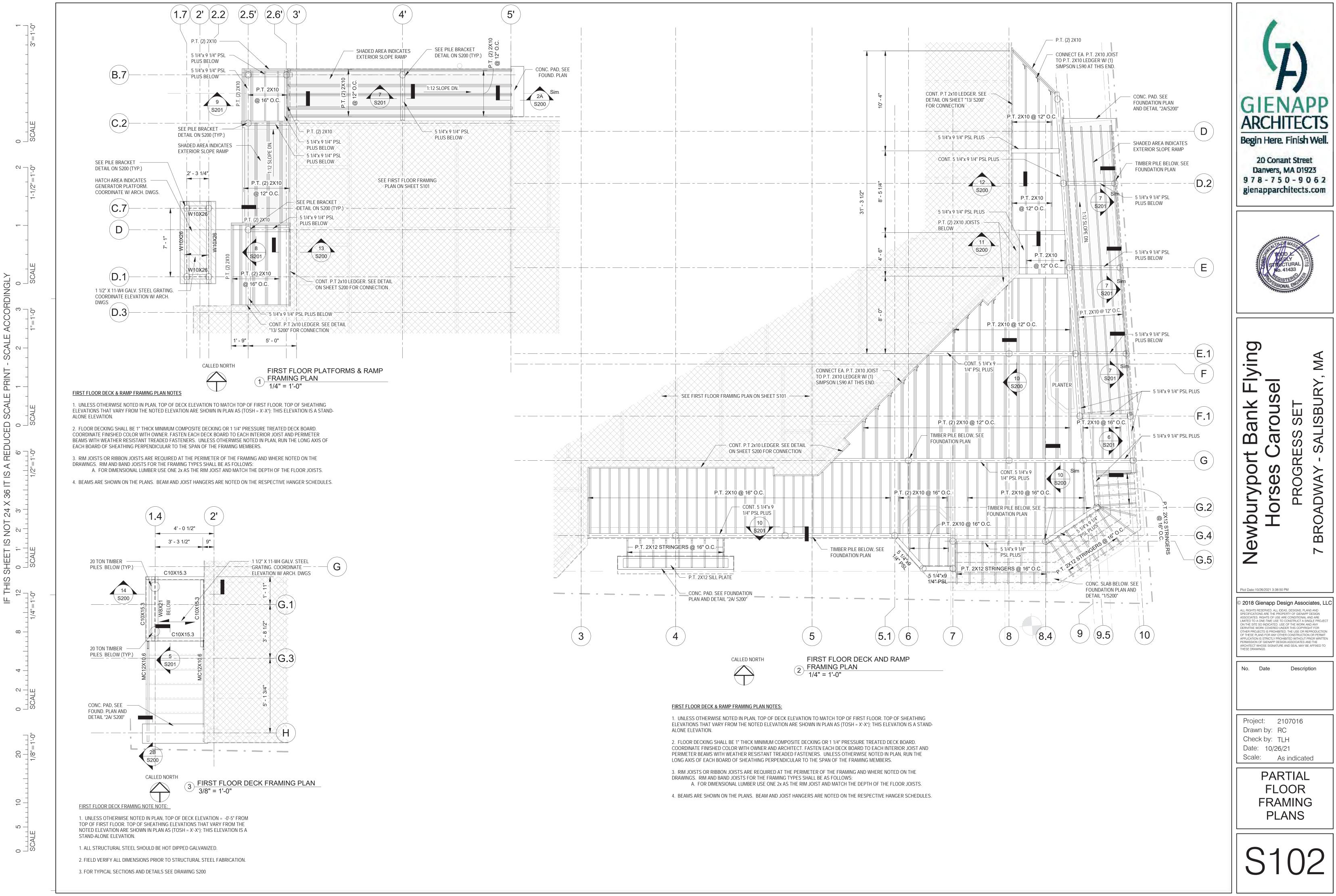


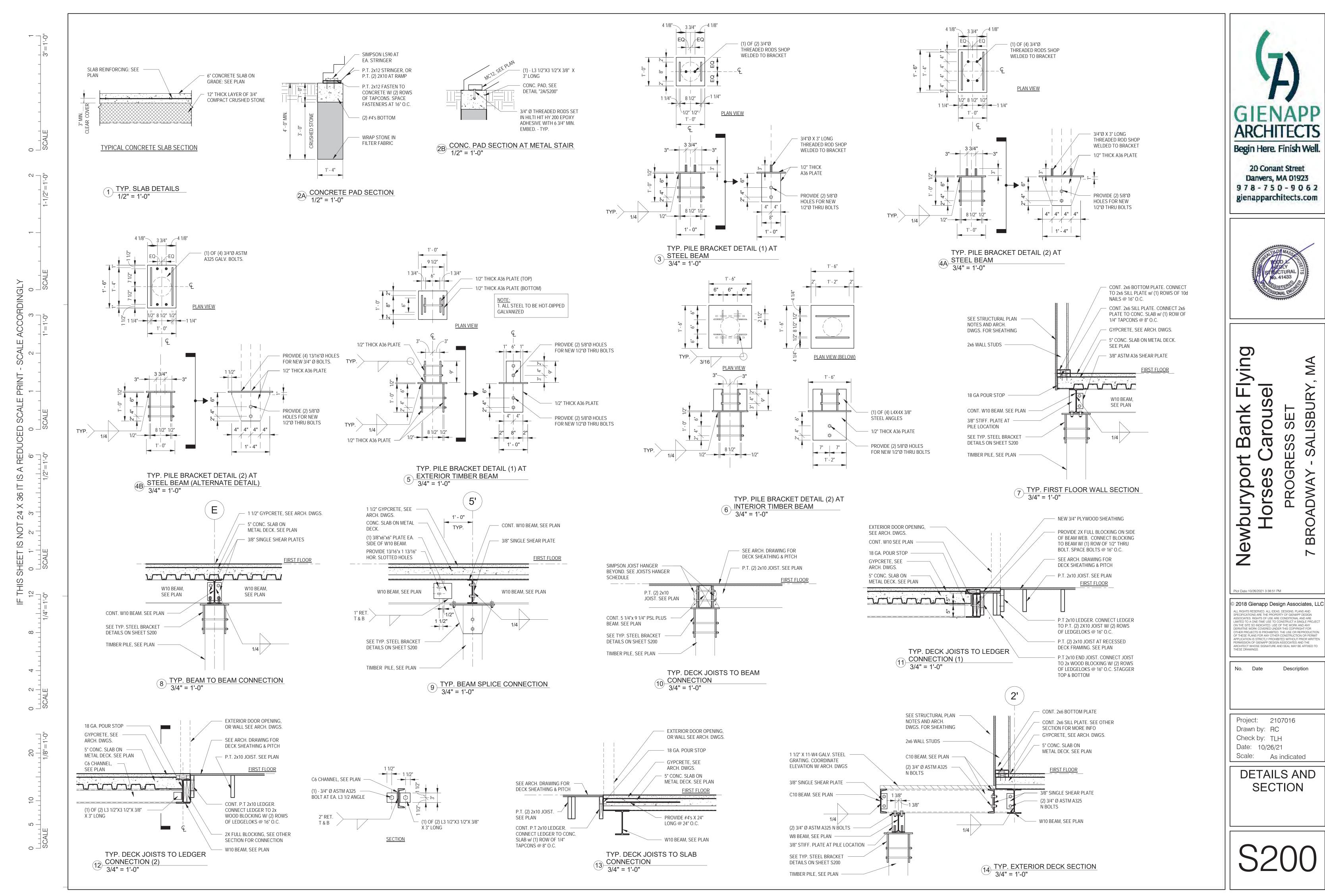












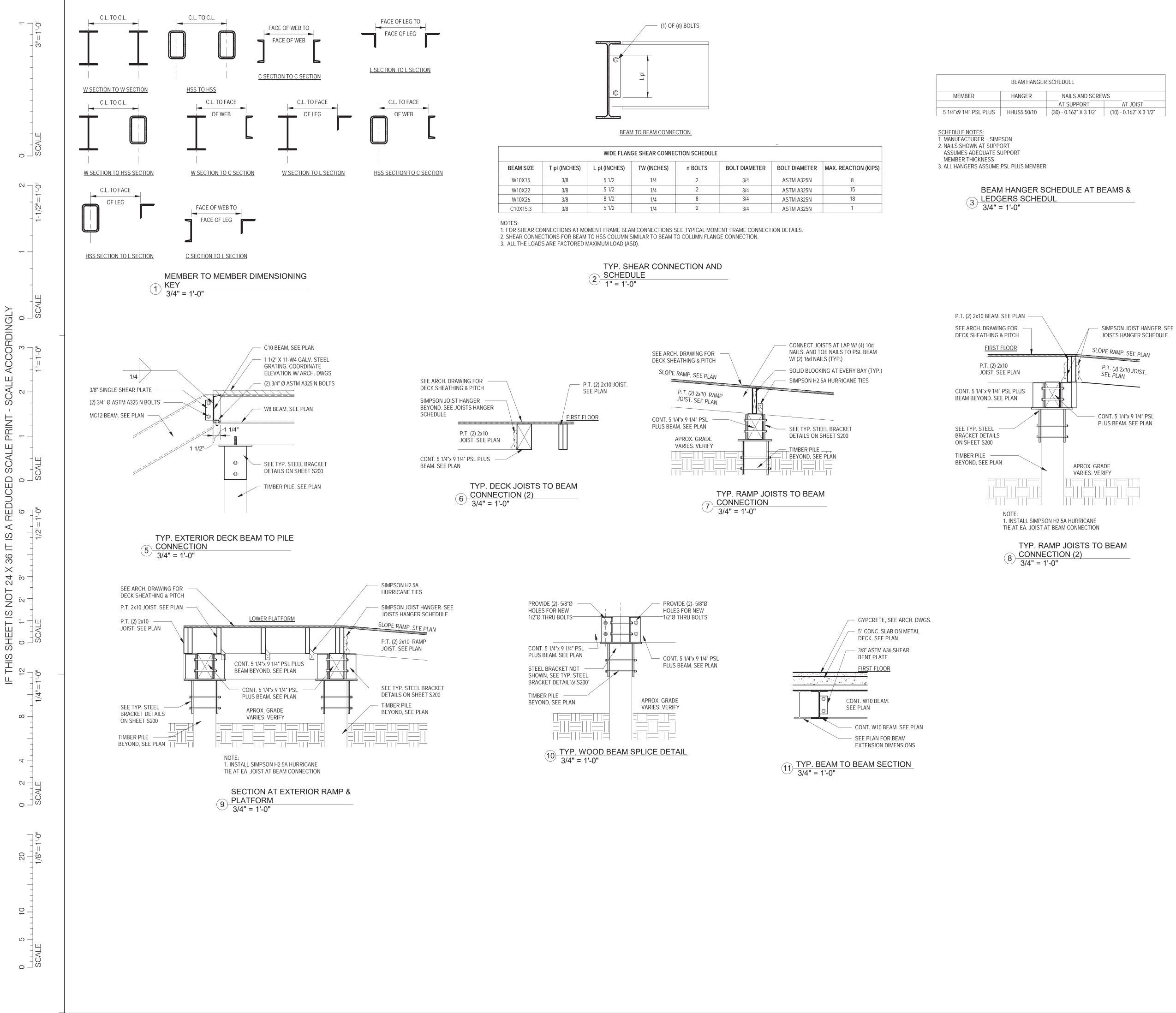
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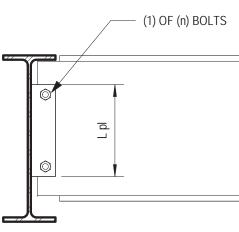
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EAM SIZE	T pl (INCHES)	L pl (INCHES)	TW (INCHES)	n BOLTS	BOLT DIAMETER	BOLT DIAMETER	MAX. REACTION (KIPS)
W10X15	3/8	5 1/2	1/4	2	3/4	ASTM A325N	8
W10X22	3/8	5 1/2	1/4	2	3/4	ASTM A325N	15
W10X26	3/8	8 1/2	1/4	8	3/4	ASTM A325N	18
C10X15.3	3/8	5 1/2	1/4	2	3/4	ASTM A325N	1

JLE	
AILS AND SCREV	VS
SUPPORT	AT JOIST
.162" X 3 1/2"	(10) - 0.162" X 3 1/2"

DIMENSIONAL LUMBER JOIST HANGER SCHEDULE AT BEAMS

MEMBER	HANGER	NAIL	S
		AT SUPPORT	AT JOIST
SINGLE 2X6	LUS26	(4) - 0.148" X 3"	(4) - 0.148" X 3"
SINGLE 2X8	LUS26	(4) - 0.148" X 3"	(4) - 0.148" X 3"
SINGLE 2X10	LUS28	(6) - 0.148" X 3"	(4) - 0.148" X 3"
SINGLE 2X12	LUS210	(8) - 0.148" X 3"	(4) - 0.148" X 3"
DOUBLE 2X6	LUS26-2	(4) - 0.162" X 3 1/2"	(4) - 0.162" X 3 1/2"
DOUBLE 2X8	LUS26-2	(4) - 0.162" X 3 1/2"	(4) - 0.162" X 3 1/2"
DOUBLE 2X10	LUS210-2	(8) - 0.162" X 3 1/2"	(4) - 0.162" X 3 1/2"
DOUBLE 2X12	LUS210-2	(8) - 0.162" X 3 1/2"	(6) - 0.162" X 3 1/2"

SCHEDULE NOTES: 1. MANUFACTURER = SIMPSON

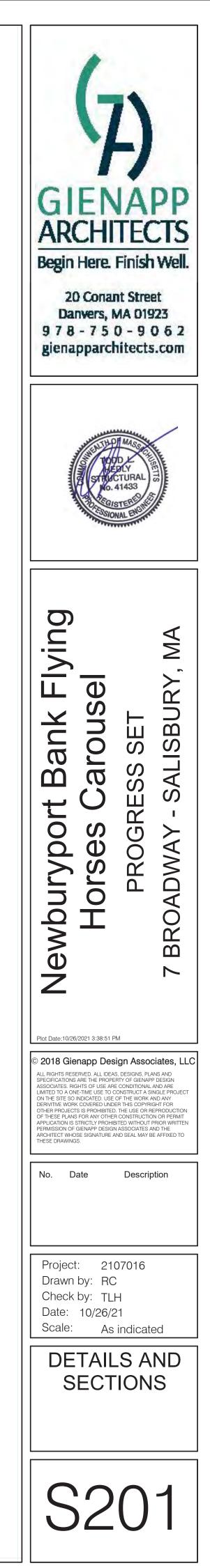
2. NAILS SHOWN AT SUPPORT ASSUMES ADEQUATE SUPPORT MEMBER THICKNESS

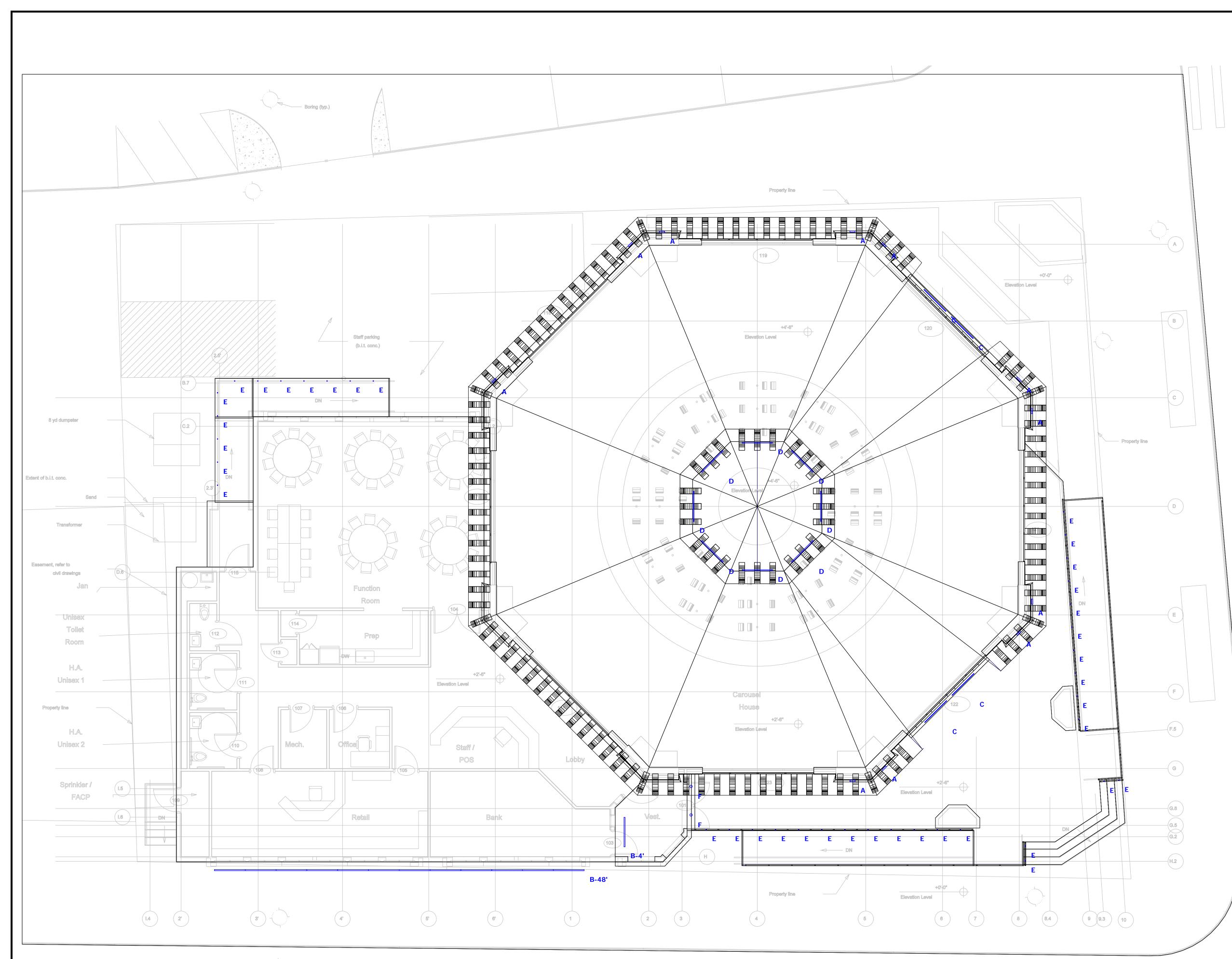
3. ALL HANGERS ASSUME SPF LUMBER 4. ALL HANGERS ARE BASED ON A LIVE LOAD OF 100 PSF

AND A DEAD LOAD OF 15 PSF

DIMENSIONAL LUMBER JOIST HANGER 4 SCHEDULE AT BEAMS & LEDGER 3/4" = 1'-0"

	X. GRADE S. VERIFY	
<u></u> _ <u>_</u> 		





Boring (typ.)



DISCLAIMER:

-THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES ONLY AND ARE NOT INTENDED FOR CONSTRUCTION. VALUES REPRESENTED ARE AN APPROXIMATION GENERATED FROM MANUFACTURERS PHOTOMETRIC IN-HOUSE OR INDEPENDANT LAB TEST WITH DATA SUPPLIED BY LAMP MANUFACTURERS.

<u>Plan View</u> Scale - 1" = 6ft

Symbol	Label	Quantity	Manufacturer	Catalog Number	Description	Light Loss Factor	Wattag
	A	11	WE-EF USA	QLS420-131-9756	QLS420 LED, Wall Luminaires / Surface Mounted - 131-9756	0.9	42
	B-4'	1	Hydrel	4750L 4FT 500LMF 30K VOLTAGE WWD MOUNTING 12" XX FINISH	4750L Linear LED Flood 4FT 500LMF 30K WWD	0.9	20.98
	B-48'	1	Hydrel	4750LCR LOP 48FT 500LMF 30K VOLTAGE WWD MOUNTING 12" XX FINISH	4750LCR Continuous Linear LED Flood 48FT 1000LMF 30K WWD	0.45	495.12
	С	4	EcoSense Lighting	L50-E-48-08-30-80-MULT- 120	Gray aluminum housing, white plastic reflector clear plastic lens enclosure	0.9	32
	D	8	LUMENPULSE	LCS2-120-48-RGBW30K-FR- XX-XX	LUMENCOVE	0.9	22
	E	38	KLIKSYSTEMS	LPOD40-30K-S-XX-XX	LPOD40-Dir-PCLens-W- SymRef-LPOD-350mA- 3000K-0.025m-451797-A	0.9	1.4
	F	2	Juno Lighting	WF4 REG SWW5 90CRI MW	5CCT 4in Regressed Wafer _ 3000K	0.9	9





Туре А



Type D



Туре Е



Type F



ISUAL



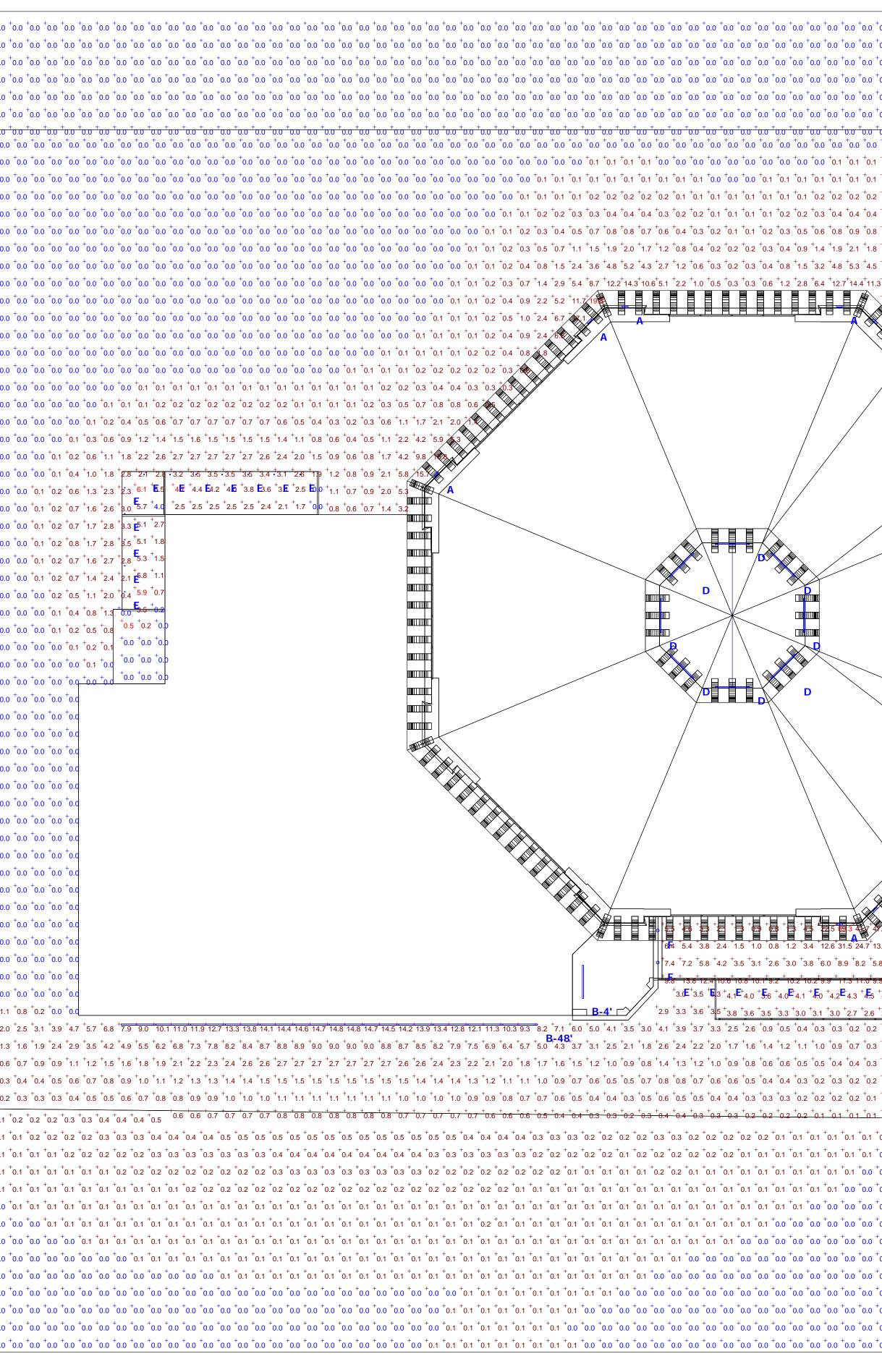
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⁺ 0.0	0.0	+	⁺ 0.0	+0.0	⁺ 0.0	+0.0	⁺ 0.0	⁺ 0.0	⁺ 0.0	+0.0	⁺ 0.0	⁺ 0.1	⁺ 0.1	⁺ 0.1	+0.1	+0.1	⁺ 0.1	⁺ 0.1	+
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NOTES:

-REFLECTANCES ASSUMED: GROUND: 20

- TASK HEIGHT: AFG - CALCULATION POINT SPACING: 2'X2' OC

SCHE	DULE						
Symbol	Label	Quantity	Manufacturer	Catalog Number	Description	Light Loss Factor	Wattage
	A	11	WE-EF USA	QLS420-131-9756	QLS420 LED, Wall Luminaires / Surface Mounted - - 131-9756	0.9	42
	B-4'	1	Hydrel	4750L 4FT 500LMF 30K VOLTAGE WWD MOUNTING 12" XX FINISH	4750L Linear LED Flood 4FT 500LMF 30K WWD	0.9	20.98
	B-48'	1	Hydrel	4750LCR LOP 48FT 500LMF 30K VOLTAGE WWD MOUNTING 12" XX FINISH	4750LCR Continuous Linear LED Flood 48FT 1000LMF 30K WWD	0.45	495.12
	С	4	EcoSense Lighting	L50-E-48-08-30-80-MULT-120	Gray aluminum housing, white plastic reflector clear plastic lens enclosure	0.9	32
	D	8	LUMENPULSE	LCS2-120-48-RGBW30K-FR-XX-XX	LUMENCOVE	0.9	22
	E	38	KLIKSYSTEMS	LPOD40-30K-S-XX-XX	LPOD40-Dir-PCLens-W-SymRef-LPOD-350mA- 3000K-0.025m-451797-A	0.9	1.4
	F	2	Juno Lighting	WF4 REG SWW5 90CRI MW	5CCT 4in Regressed Wafer _ 3000K	0.9	9



DISCLAIMER:

Plan	Vie	W
Scale -	1" :	= 8ft

-THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES ONLY AND ARE NOT INTENDED FOR CONSTRUCTION. VALUES REPRESENTED ARE AN APPROXIMATION GENERATED FROM MANUFACTURERS PHOTOMETRIC IN-HOUSE OR INDEPENDANT LAB TEST WITH DATA SUPPLIED BY LAMP MANUFACTURERS.

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	3.3 4.7 0.5 4.7 3.4 1.3	+0.5 +0.2 +0.1 +0.0 +0.0 +0.0 +0.0 +0.	0.0 0.0 0.0 0.0 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0		
	$\begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} $	+0.4 +0.2 +0.0 +0.0 +0.0 +0.0 +0.0 +0. +0.0 +0.2 +0.1 +0.1 +0.0 +0.0 +0.	0.0 0.0 0.0 0.0 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0		⁺ 0.0
		+0.0 +0.3 +0.1 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0.0 0.0 0.0 0.0 0.0	$^{+}0.0$ $^{+}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{+}0.0$ $^{+}0.0$ $^{+}0.0$ $^{+}0.0$ $^{+}0.0$ $^{+}0.0$ $^{+}0.0$		$^{+}0.0$ $^{+}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{-}0.0$ $^{+}$
	$5\mathbf{n}^{+}23.5^{+}13.6\mathbf{f}_{5.2}^{+}3.8^{+}.0$ $37.5^{+}12.7^{+}1.9_{5.3}^{+}3.5^{+}1.6$ $5\mathbf{n}^{+}26.4^{+}7.6^{+}9.4^{+}.5^{+}.5^{+}1.6$	+0.0 +0.2 +0.1 +0.0 +0.0 +0.0 +0.0 +0.	0 ^{+0.0+} 0.0 ^{+0.0} +0.0 ^{+0.0} +0.0 ^{+0.0} +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+ + + + + + +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	5.7 3.1 0.4	$^{+}0.0$ $^{+}0.1$ $^{+}0.1$ $^{+}0.1$ $^{+}0.1$ $^{+}0.0$ $^{+}$	0.0 0.0 0.0 0.0 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	$5^{+}12.0^{+}8.0^{+}3.8^{+}8.6^{+}E_{0}^{+}2.7^{+}0.4$ 9 $^{+}2.5^{+}2.1^{+}1.7^{+}6.7^{+}6.7^{+}6.7^{+}2.4^{+}0.4$	$ \begin{array}{c} {}^{+}0.0 & {}^{+}0.0 & {}^{+}0.1 & {}^{+}0.1 & {}^{+}0.0 $		⁺ 0.0	0 ⁺ 0.0	⁺ 0.0 ⁺
0.1 +0.2 +0.4 +0.4	0.7 2.4	$\frac{2}{2}^{+}$ 0.0 $^{+}$ 0.		+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+ + + + + + + +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
				+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
0.2 ⁺ 0.2 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0. 0.0 ⁺ 0.7 ⁺ 0.3 ^C 0.1 ⁺ 0.1 ⁺ 0.0 ⁺ 0.0	$1^{+}0.1^{+}0.1^{+}0.1^{+}0.1^{+}0.0^{+}0.0^{+}1.1^{+}$	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0. +0.0 +0.0	+	⁺ 0.0	0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	⁺ 0.0 ⁺
+4.9 +1.3 +0.4 +0.1 +0.1 +0.0 +0.1	0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.1 ⁺ 0.0 ⁺ 0.1 ⁺ 0.	⁺ 0.1 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.	$\begin{array}{c} 0.0 & 0.0 & 0.0 & 0.0 \\ 0 & 0.0 & 0.0 & 0.0 \\ 0.0 & 0.0 & 0.0 & 0.0 \\ \end{array}$	+ + + + + + +	0 ⁺ 0.0	
8.9 ⁺ 8.9 ⁺ 1.9 ⁺ 0.5 ⁺ 0.2 ⁺ 0.1 ⁺ 0.0 ⁺ 0.0	0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0	1.7 + 0.3 + 0.0		+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
*128.8 ⁺ 7.3 ⁺ 1.8 ⁺ 0.6 ⁺ 0.2 ⁺ 0.1 ⁺ 0.0 ⁺ 0.0	0 ⁺ 0.0	$\begin{bmatrix} 0.6 & 0.0 & 0.0 & 0.0 & 0.0 \\ 15.0 \\ + \end{bmatrix} \begin{bmatrix} \mathbf{E} & + \\ 1.8 & + \\ 0.2 & + \\ 0.0 & + \\ 0.0 & - \\ 0.0 & + \\ 0.0 & - \\ 0.0 & + \\ 0.0 & - \\ 0.0 & + \\ 0.0 & - \\ 0.0 & + \\ 0.0 & - \\ 0.0 & + \\ 0.0 & - $		⁺ 0.0	0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 0 ⁺ 0.0	
+5.8 $+4.5$ $+3.4$ $+2.5$ $+0.3$ $+0.0$ $+0.0$ $+0.0$ $+0.0$ $+0.0$	0 ⁺ 0.0	+ 0.4 + 0.5 + 0.1 + 0.0 + 0.	0 ⁺ 0.0 ⁺ 0.	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	
10.5⁺ 9.9 ⁺ 8.9 ⁺ 0.0 		+ + 0.1 + 0.0 +	+ +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
$5^{+}4.8^{\pm}5.0^{+}9.3^{+}5.3^{\pm}5.3^{+}4.3^{+}$	$0^{+} + \frac{1}{2.8} + \frac{2}{0.7} + \frac{1}{0.0} + \frac{1}{0.0$	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	0 0.0 0.0 + 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 ⁺ 0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0
	$\begin{array}{c} \underline{0} & \underline{0} & \underline{12.3} & 0.6 & 0.0 \\ \hline & 3.0 & 5 \\ \underline{5} & 1.8 & 0.3 & 0.1 & 0.0 & 0.0 \end{array}$	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.	$0^{+0.0}$ $+0.0^{+0.0}$ $+0.0^{+0.0}$ $+0.0^{+0.0}$ $+0.0^{+0.0}$	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 +0.0 +0.0	0 ⁺ 0.0	
.3 +0.2 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.3 +0.1 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.	+ 0.0 0.0 0.0 0.0 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
$.3 \ ^{+}0.3 \ ^{+}0.2 \ ^{+}0.2 \ ^{+}0.1 \ ^{+}0.1 \ ^{+}0.1 \ ^{+}0.1 \ ^{+}0.1 \ ^{+}0.0 \ ^{+}0.0$	$^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$ $^{\circ}0.0$	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	$0^{+}0.$	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.0 ⁺ 0.0			⁺ 0.0	⁺ 0.0 ⁺	0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0 ⁺ 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
<u>1 + 0.1 + 0.1 + 0.1 + 0.0 + 0.0 + 0.0 + 0.0 + 0.0 + 0.0</u>	+ <u>0.0</u> + <u></u>	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
+ + + + + + + + + + + + + + + + + + +	0.0 ⁺	0.0 + 0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 + + + +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 ⁺	0.0 ⁺ 0.0	⁺ 0.0	⁺ 0.0 ⁺	o ⁺ 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
) ⁺ 0.0	0.0 ⁺	0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+ + + + + + + +	0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
+ + + + + + + + + + + + + + + + + + +	0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +	0.0 + 0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	$0.0 \ 0.0 $	0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	$^{+}$ 00 $^$	$0 \ 0.0 \ $	$\begin{array}{c} 0.0 &$
) ⁺ 0.0	-0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
						+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
						*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0
						0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
						+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0

STATISTIC	CS					
DESCRIPTION	SYMBOL	AVG.	MAX	MIN.	MAX/MIN	AVG/MIN
Back Ramp	+	3.2 fc	5.9 fc	0.2 fc	29.5:1	16.0:1
Back Ramp	+	2.7 fc	4.7 fc	0.0 fc	N/A	N/A
Back Ramp	+	5.3 fc	6.1 fc	4.0 fc	1.5:1	1.3:1
Back Ramp	+	0.1 fc	0.5 fc	0.0 fc	N/A	N/A
Boundary	+	0.0 fc	0.6 fc	0.0 fc	N/A	N/A
Deck	+	5.2 fc	69.3 fc	0.0 fc	N/A	N/A
Property	+	0.7 fc	19.8 fc	0.0 fc	N/A	N/A
Ramp	+	3.5 fc	5.3 fc	0.6 fc	8.8:1	5.8:1
Ramp	+	2.9 fc	7.5 fc	0.0 fc	N/A	N/A
Stair	+	4.1 fc	13.7 fc	0.0 fc	N/A	N/A
Stair	+	0.6 fc	3.2 fc	0.0 fc	N/A	N/A
Stair	+	3.9 fc	15.0 fc	0.0 fc	N/A	N/A



ISUAL

